MODE Advanced Vibrations

Lecture One

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UMASS LOWELL MODAL ANALYSIS and CONTROLS LABORATORY - Pete Avitabile and Fabio Piergentili

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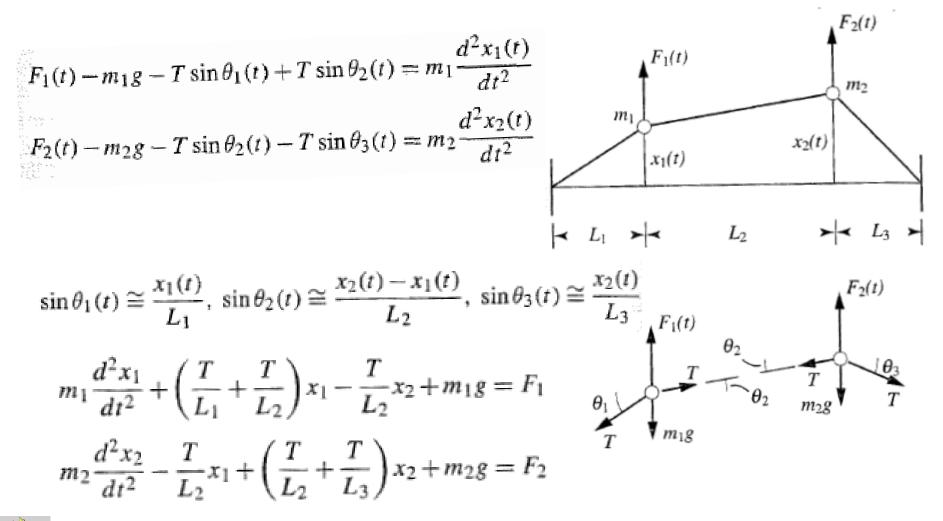
MODE

Preliminaries: Multi-Degree-of-Freedom Systems

- 1. THE EQUATION OF MOTION
- 2. FREE VIBRATIONS
- 3. EIGEN PROBLEM
- 4. MODE SHAPES
- 5. RESPONSE TO INITIAL EXCITATIONS
- 6. COORDINATE TRANSFORMATION
- 7. ORTHOGONALITY OF MODES, NATURAL COORDINATES
- 8. BEAT PHENOMENON

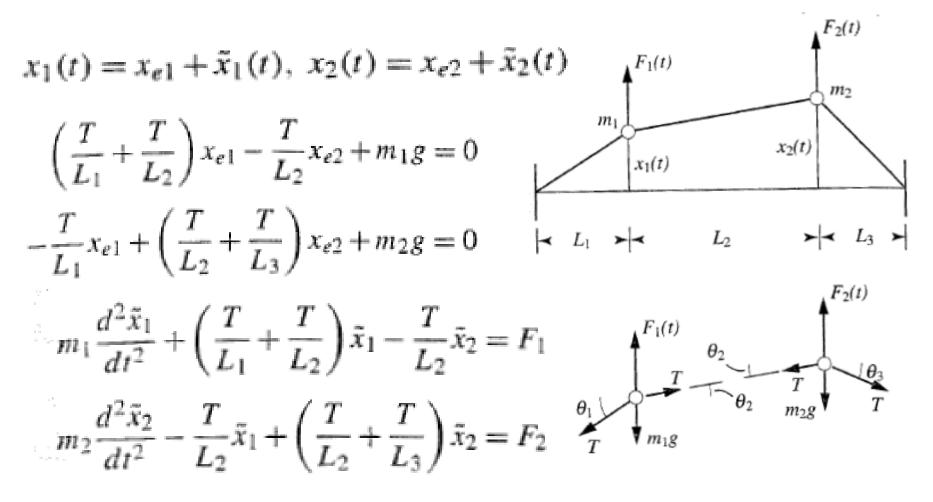


THE EQUATION OF MOTION OF MULTI DEGREE OF FREEIDOM SYSTEMS



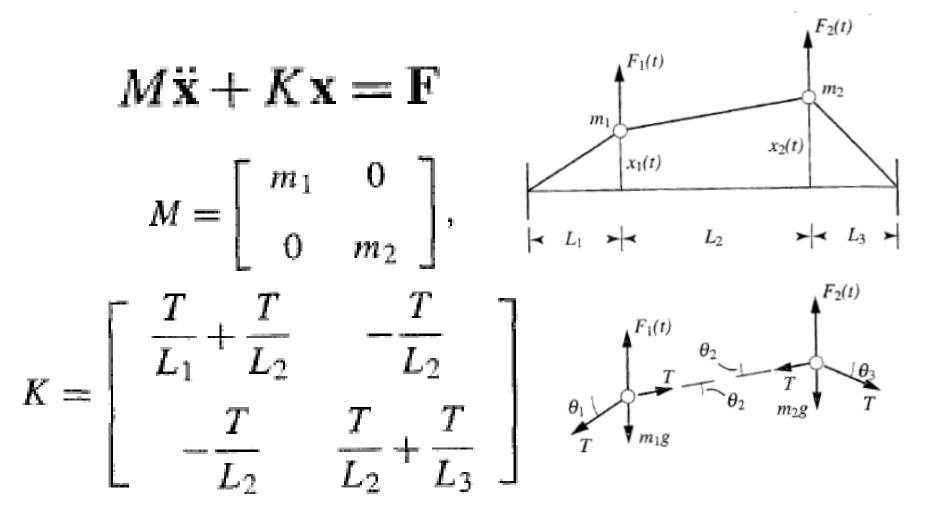


THE EQUATION OF MOTION OF MULTI DEGREE OF FREEIDOM SYSTEMS





THE EQUATION OF MOTION OF MULTI DEGREE OF FREEIDOM SYSTEMS





FREE VIBRATIONS OF UNDAMPED SYSTEMS, NATURAL MODES $M\ddot{\mathbf{x}}(t) + K\mathbf{x}(t) = \mathbf{0}$ $\tilde{f}(t)M\mathbf{u} + f(t)K\mathbf{u} = \mathbf{0}$ $\lambda = \omega^2$ $\ddot{f}(t)\mathbf{u}^T M\mathbf{u} + f(t)\mathbf{u}^T K\mathbf{u} = 0$ $\ddot{f}(t) + \omega^2 f(t) = 0$ $\frac{\mathbf{u}^T K \mathbf{u}}{\mathbf{u}^T M \mathbf{u}} = \lambda$ $f(t) = C\cos(\omega t - \phi)$ $\ddot{f}(t) + \lambda f(t) = 0$ $K\mathbf{u} = \lambda M\mathbf{u}$

EIGEN PROBLEM

$$K\mathbf{u} = \lambda M\mathbf{u} \qquad (k_{11} - \omega^2 m_1)u_1 + k_{12}u_2 = 0$$

$$k_{12}u_1 + (k_{12} - \omega^2 m_2)u_2 = 0$$

$$\Delta(\omega^2) = \det \begin{bmatrix} k_{11} - \omega^2 m_1 & k_{12} \\ k_{12} & k_{22} - \omega^2 m_2 \end{bmatrix} = 0$$

$$\Delta\omega^2 = m_1 m_2 \begin{bmatrix} \omega^4 - \left(\frac{k_{11}}{m_1} + \frac{k_{22}}{m_2}\right)\omega^2 + \frac{k_{11}k_{22} - k_{12}^2}{m_1 m_2} \end{bmatrix} = 0$$

$$\frac{\omega_1^2}{\omega_2^2} = \frac{1}{2} \left(\frac{k_{11}}{m_1} + \frac{k_{22}}{m_2}\right) \mp \frac{1}{2} \sqrt{\left(\frac{k_{11}}{m_1} + \frac{k_{22}}{m_2}\right)^2 - 4\frac{k_{11}k_{22} - k_{12}^2}{m_1 m_2}}$$

$$f_1(t) = C_1 \cos(\omega_1 t - \phi_1), \quad f_2(t) = C_2 \cos(\omega_2 t - \phi_2)$$



MODE SHAPES

$$K\mathbf{u} = \lambda M\mathbf{u} \qquad (k_{11} - \omega_i^2 m_1)u_{1i} + k_{12}u_{2i} = 0 \\ k_{12}u_{1i} + (k_{22} - \omega_i^2 m_2)u_{2i} = 0 \end{cases} i = 1, 2$$

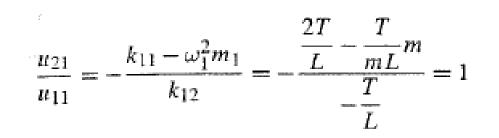
$$\left\{ \frac{u_{21}}{u_{11}} = -\frac{k_{11} - \omega_1^2 m_1}{k_{12}} = -\frac{k_{12}}{k_{22} - \omega_1^2 m_2} \\ \frac{u_{22}}{u_{12}} = -\frac{k_{11} - \omega_2^2 m_1}{k_{12}} = -\frac{k_{12}}{k_{22} - \omega_2^2 m_2} \\ \mathbf{u}_1 = \begin{bmatrix} u_{11} \\ u_{21} \end{bmatrix}, \ \mathbf{u}_2 = \begin{bmatrix} u_{12} \\ u_{22} \end{bmatrix} \qquad \mathbf{x}_1(t) = f_1(t)\mathbf{u}_1 = C_1\mathbf{u}_1\cos(\omega_1 t - \phi_1) \\ \mathbf{x}_2(t) = f_2(t)\mathbf{u}_2 = C_2\mathbf{u}_2\cos(\omega_2 t - \phi_2)\mathbf{u}_2 \\ \mathbf{x}(t) = \mathbf{x}_1(t) + \mathbf{x}_2(t) = C_1\cos(\omega_1 t - \phi_1)\mathbf{u}_1 + C_2\cos(\omega_2 t - \phi_2)\mathbf{u}_2$$

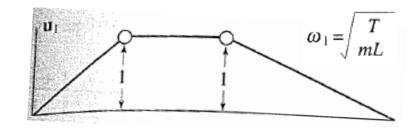


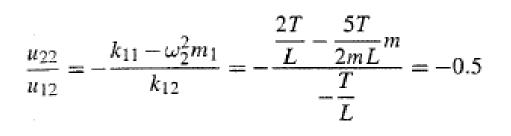
EXAMPLE

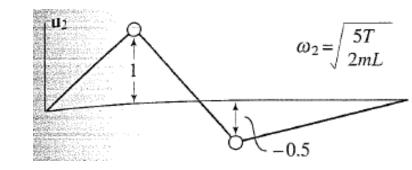
 $\downarrow^{F_2(t)}$

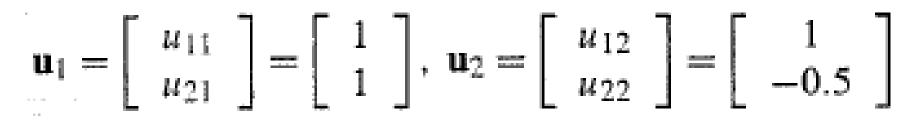
EXAMPLE













RESPONSE TO INITIAL EXCITATIONS

$$\mathbf{x}(0) = \begin{bmatrix} x_{10} \\ x_{20} \end{bmatrix}, \ \dot{\mathbf{x}}(0) = \mathbf{v}(0) = \begin{bmatrix} v_{10} \\ v_{20} \end{bmatrix}$$

$$x_{10} = u_{11}C_1 \cos \phi_1 + u_{12}C_2 \cos \phi_2$$

$$x_{20} = u_{21}C_1 \cos \phi_1 + u_{22}C_2 \cos \phi_2$$

$$v_{10} = \omega_1 u_{11}C_1 \sin \phi_1 + \omega_2 u_{12}C_2 \sin \phi_2$$

$$v_{20} = \omega_1 u_{21}C_1 \sin \phi_1 + \omega_2 u_{22}C_2 \sin \phi_2$$

$$C_1 \cos \phi_1 = \frac{u_{22}x_{10} - u_{12}x_{20}}{|U|}, \ C_2 \cos \phi_2 = \frac{u_{11}x_{20} - u_{21}x_{10}}{|U|}$$

$$C_1 \sin \phi_1 = \frac{u_{22}v_{10} - u_{12}v_{20}}{\omega_1|U|}, \ C_2 \sin \phi_2 = \frac{u_{11}v_{20} - u_{21}v_{10}}{\omega_2|U|}$$
where $|U|$ is the determinant of the modal matrix U ,

$$U = \begin{bmatrix} \mathbf{u}_1 & \mathbf{u}_2 \end{bmatrix} = \begin{bmatrix} u_{11} & u_{12} \\ u_{21} & u_{22} \end{bmatrix}$$

EXAMPLE

 $X_1(0) = 1.2$ cm. The other initial conditions are zero.

$$\begin{split} \omega_{1} &= \sqrt{\frac{T}{mL}}, \ \omega_{2} = 1.581139 \sqrt{\frac{T}{mL}} \ (rad/s) \\ U &= \begin{bmatrix} u_{11} & u_{12} \\ u_{21} & u_{22} \end{bmatrix} = \begin{bmatrix} 1 & 1 \\ 1 & -0.5 \end{bmatrix} \\ |U| &= u_{11}u_{22} - u_{12}u_{21} = -1.5 \\ \mathbf{x}(t) &= \begin{bmatrix} x_{1}(t) \\ x_{2}(t) \end{bmatrix} = \frac{1}{-1.5} \begin{cases} (-0.5) \times 1.2 \cos \sqrt{\frac{T}{mL}} t \begin{bmatrix} 1 \\ 1 \end{bmatrix} \\ -1 \times 1.2 \cos 1.581139 \sqrt{\frac{T}{mL}} t \begin{bmatrix} 1 \\ -0.5 \end{bmatrix} \end{cases} \\ &= \begin{bmatrix} 0.4 \cos \sqrt{\frac{T}{mL}} t + 0.8 \cos 1.581139 \sqrt{\frac{T}{mL}} t \\ 0.4 \cos \sqrt{\frac{T}{mL}} t - 0.4 \cos 1.581139 \sqrt{\frac{T}{mL}} t \end{bmatrix}$$
(cm)

COORDINATE TRANSFORMATION, COUPLING $x_2(t)$ $M\ddot{\mathbf{x}}(t) + K\mathbf{x}(t) = \mathbf{0} \qquad \begin{bmatrix} k_1 \\ m_1 \end{bmatrix} \qquad m_1$ m_2 $M = \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix}, K = \begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix}$ $x_1(t) = z_1(t), x_2(t) = z_1(t) + z_2(t)$ $\mathbf{x}(t) = T\mathbf{z}(t) \qquad M'\ddot{\mathbf{z}}(t) + K'\mathbf{z}(t) = \mathbf{0}$ $T = \begin{bmatrix} 1 & 0 \\ 1 & 1 \end{bmatrix} \begin{bmatrix} M' = \begin{bmatrix} 1 & 1 \\ 0 & 1 \end{bmatrix} \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{bmatrix} 1 & 0 \\ 1 & 1 \end{bmatrix} = \begin{bmatrix} m_1 + m_2 & m_2 \\ m_2 & m_2 \end{bmatrix}$ $K' = \begin{bmatrix} 1 & 1 \\ 0 & 1 \end{bmatrix} \begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix} \begin{bmatrix} 1 & 0 \\ 1 & 1 \end{bmatrix} = \begin{bmatrix} k_1 & 0 \\ 0 & k_2 \end{bmatrix}$

ORTHOGONALITY OF MODES, NATURAL COORDINATES

$$K\mathbf{u}_{1} = \omega_{1}^{2} M \mathbf{u}_{1}$$

$$K\mathbf{u}_{2} = \omega_{2}^{2} M \mathbf{u}_{2}$$

$$\mathbf{x}(t) = q_{1}(t)\mathbf{u}_{1} + q_{2}(t)\mathbf{u}_{2}$$

$$\ddot{q}_{1}(t) + \omega_{1}^{2}q_{1}(t) = 0$$

$$\ddot{q}_{2}(t) + \omega_{2}^{2}q_{2}(t) = 0$$

$$q_{1}(t) = C_{1}\cos(\omega_{1}t - \phi_{1}), \ q_{2}(t) = C_{2}\cos(\omega_{2}t - \phi_{2})$$

$$\mathbf{x}(t) = C_{1}\cos(\omega_{1}t - \phi_{1})\mathbf{u}_{1} + C_{2}\cos(\omega_{2}t - \phi_{2})\mathbf{u}_{2}$$

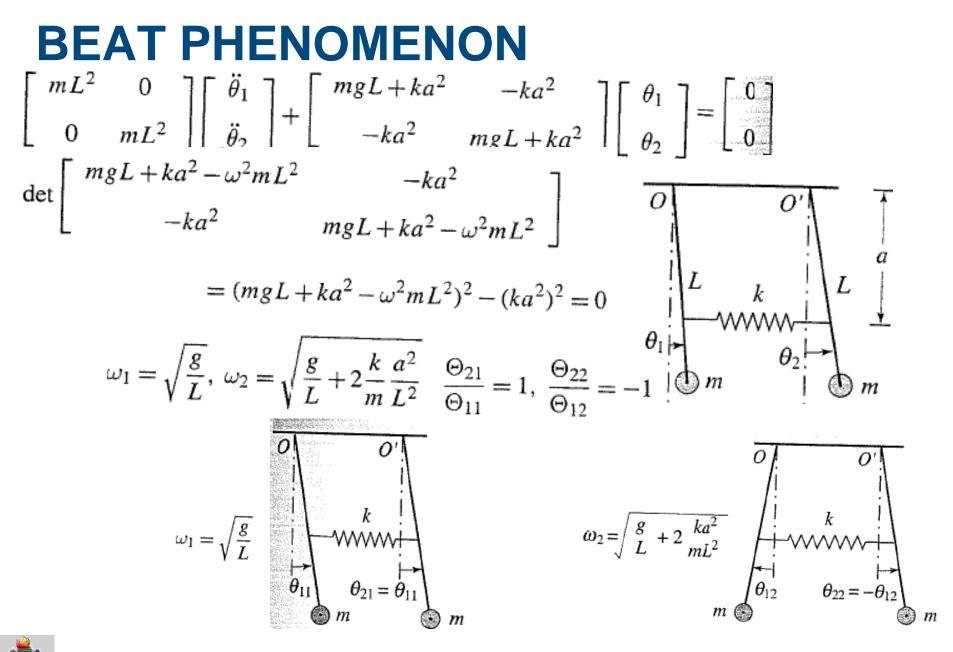


EXAMPLE

$$M\ddot{\mathbf{x}}(t) + K\mathbf{x}(t) = \mathbf{0}$$

 $M = \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} = m \begin{bmatrix} 1 & 0 \\ 0 & 2 \end{bmatrix} \mathbf{x}(0) = \begin{bmatrix} 1.2 \\ 0 \end{bmatrix}, \dot{\mathbf{x}}(0) = \mathbf{0}$
 $K = \begin{bmatrix} k_{11} & k_{12} \\ k_{12} & k_{22} \end{bmatrix} = \frac{T}{L} \begin{bmatrix} 2 & -1 \\ -1 & 3 \end{bmatrix}$
 $\omega_1 = \sqrt{\frac{T}{mL}}, \ \omega_2 = \sqrt{\frac{5T}{2mL}} = 1.581139 \sqrt{\frac{T}{mL}} \quad \mathbf{u}_1 = \begin{bmatrix} 1 \\ 1 \end{bmatrix}, \ \mathbf{u}_2 = \begin{bmatrix} 1 \\ -0.5 \end{bmatrix}$
Introducing the linear transformation:
 $\mathbf{x}(t) = q_1(t)\mathbf{u}_1 + q_2(t)\mathbf{u}_2 = q_1(t) \begin{bmatrix} 1 \\ 1 \end{bmatrix} + q_2(t) \begin{bmatrix} 1 \\ -0.5 \end{bmatrix} \quad \ddot{q}_1(t) + \sqrt{\frac{T}{mL}}q_1(t) = \mathbf{0}$
 $\ddot{q}_2(t) + \sqrt{\frac{5T}{2mL}}q_2(t) = \mathbf{0}$
 $\mathbf{x}(0) = \begin{bmatrix} 1.2 \\ 0 \end{bmatrix} = q_1(0)\mathbf{u}_1 + q_2(0)\mathbf{u}_2 = q_1(0) \begin{bmatrix} 1 \\ 1 \end{bmatrix} + q_2(0) \begin{bmatrix} 1 \\ -0.5 \end{bmatrix} q_1(0) = 0.4, \ q_2(0) = 0.8$
 $q_1(t) = q_1(0)\cos\omega_1 t = 0.4\cos\sqrt{\frac{T}{mL}}t \quad q_2(t) = q_2(0)\cos\omega_2 t = 0.8\cos 1.581139\sqrt{\frac{T}{mL}}t$
 $\ddot{\mathbf{x}}(t) = 0.4\cos\sqrt{\frac{T}{mL}}t \begin{bmatrix} 1 \\ 1 \end{bmatrix} + 0.8\cos 1.581139\sqrt{\frac{T}{mL}}t \begin{bmatrix} 1 \\ -0.5 \end{bmatrix}$





BEAT PHENOMENON

Then, considering the initial conditions: $\theta_1(0) = \theta_0, \ \theta_2(0) = 0, \ \theta_1(0) = \theta_2(0) = 0$ $\theta_1(t) = \frac{1}{2}\theta_0 \cos \omega_1 t + \frac{1}{2}\theta_0 \cos \omega_2 t = \theta_0 \cos \frac{\omega_2 - \omega_1}{2} t \cos \frac{\omega_2 + \omega_1}{2} t$ $\theta_2(t) = \frac{1}{2}\theta_0 \cos \omega_1 t - \frac{1}{2}\theta_0 \cos \omega_2 t = \theta_0 \sin \frac{\omega_2 - \omega_1}{2} t \sin \frac{\omega_2 + \omega_1}{2} t$ $k \ll mgL/a^2$ $\frac{\omega_B}{2} = \frac{\omega_2 - \omega_1}{2} \cong \frac{1}{2} \frac{k}{m} \frac{a^2}{\sqrt{gL^3}},$ $\omega_{\text{ave}} = \frac{\omega_2 + \omega_1}{2} \cong \sqrt{\frac{g}{L}} + \frac{1}{2} \frac{k}{m} \frac{a^2}{\sqrt{gL^3}}$ $\theta_2(t)$ $\theta_1(t) \cong \theta_0 \cos \frac{1}{2} \omega_B t \cos \omega_{\text{ave}} t$, $\theta_2(t) \cong \theta_0 \sin \frac{1}{2} \omega_B t \sin \omega_{ave} t$

WE COVERED: Multi-Degree-of-Freedom Systems

- 1. THE EQUATION OF MOTION
- 2. FREE VIBRATIONS
- 3. EIGEN PROBLEM
- 4. MODE SHAPES
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MODE Advanced Vibrations Lecture Two MODE 4 MODE 2 By: H. Ahmadian ahmadian@iust.ac.ir



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Preliminaries: Multi-Degree-of-Freedom Systems

- 1. RESPONSE TO HARMONIC EXCITATIONS
- **2. UNDAMPED VIBRATION ABSORBERS**
- 3. RESPONSE TO NONPERODIC EXCITATIONS

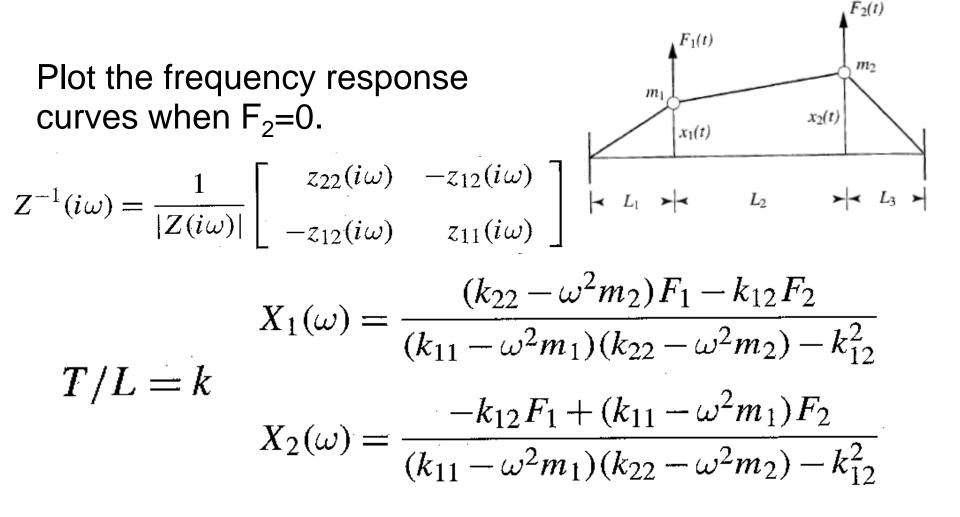


RESPONSE TO HARMONIC EXCITATIONS

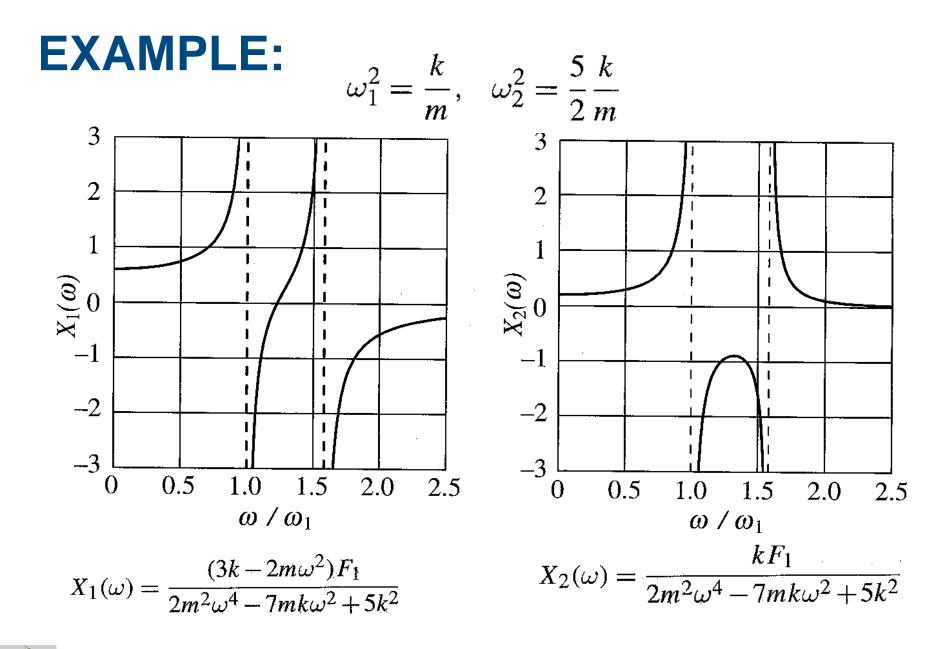
 $M\ddot{\mathbf{x}}(t) + C\dot{\mathbf{x}}(t) + K\mathbf{x}(t) = \mathbf{F}(t)$ $\mathbf{F}(t) = \mathbf{F}e^{i\omega t}$ $\mathbf{x}(t) = \mathbf{X}(i\omega)e^{i\omega t}$ $Z(i\omega)\mathbf{X}(i\omega) = \mathbf{F}$ $Z(i\omega) = -\omega^2 M + i\omega C + K$ $\mathbf{X}(i\omega) = Z^{-1}(i\omega)\mathbf{F}$



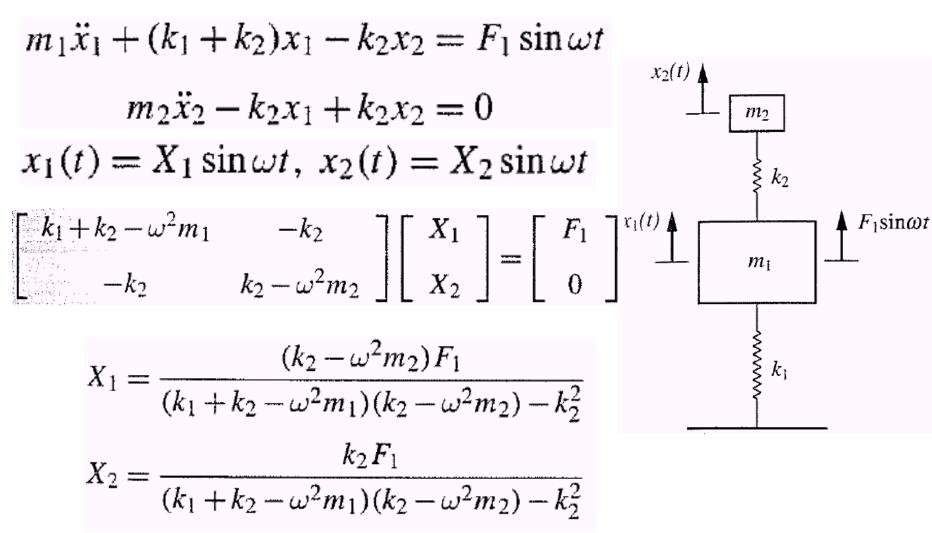
EXAMPLE:

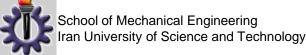






UNDAMPED VIBRATION ABSORBERS





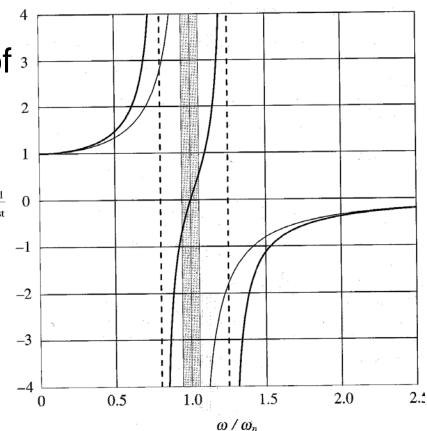
UNDAMPED VIBRATION ABSORBERS

The shaded area indicates the ⁴ region which the performance of ³ the absorber can be regarded ² as satisfactory. ¹

One disadvantage of the $\frac{X_1}{x_{st}}$ vibration absorber is that two new resonance frequencies are created.

$$\omega_n = \sqrt{k_1/m_1} \quad x_{st} = F_1/k_1$$
$$\omega_a = \sqrt{k_2/m_2} \quad \mu = m_2/m_1$$

$$\mu = 0.2$$
 and $\omega_n = \omega_a$.





RESPONSE TO NON-PERIODIC EXCITATIONS

$$M\ddot{\mathbf{x}}(t) + K\mathbf{x}(t) = \mathbf{F}(t)$$

$$M = \begin{bmatrix} m_{11} & m_{12} \\ m_{12} & m_{22} \end{bmatrix}, K = \begin{bmatrix} k_{11} & k_{12} \\ k_{12} & k_{22} \end{bmatrix}$$
Modal
Coordinates
$$\mathbf{x}(t) = \eta_1(t)\mathbf{u}_1 + \eta_2(t)\mathbf{u}_2$$

$$m'_{11}\ddot{\eta}_1(t) + m'_{11}\omega_1^2\eta_1(t) = N_1(t)$$

$$m'_{22}\ddot{\eta}_2(t) + m'_{22}\omega_2^2\eta_2(t) = N_2(t)$$

$$N_1(t) = \mathbf{u}_1^T\mathbf{F}(t), N_2(t) = \mathbf{u}_2^T\mathbf{F}(t)$$



RESPONSE TO NON-PERIODIC EXCITATIONS

$$\eta_1(t) = \int_0^t N_1(t-\tau)g_1(\tau)d\tau = \frac{1}{m'_{11}\omega_1} \int_0^t N_1(t-\tau)\sin\omega_1\tau d\tau$$
$$\eta_2(t) = \int_0^t N_2(t-\tau)g_2(\tau)d\tau = \frac{1}{m'_{22}\omega_2} \int_0^t N_2(t-\tau)\sin\omega_2\tau d\tau$$

$\mathbf{x}(t) = \eta_1(t)\mathbf{u}_1 + \eta_2(t)\mathbf{u}_2$



EXAMPLE:

$$F_{2}(t) = F_{0}[w(t) - w(t - a)]$$

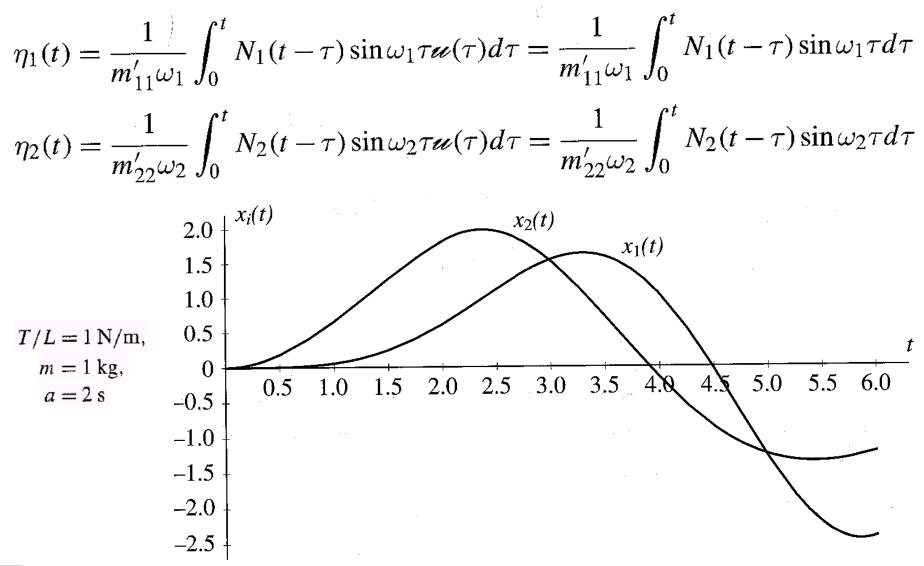
$$M = m \begin{bmatrix} 1 & 0 \\ 0 & 2 \end{bmatrix}, \quad K = \frac{T}{L} \begin{bmatrix} 2 & -1 \\ -1 & 3 \end{bmatrix}$$

$$\omega_{1} = \sqrt{\frac{T}{mL}}, \quad \omega_{2} = \sqrt{\frac{5T}{2mL}} \quad \mathbf{u}_{1} = \begin{bmatrix} 1 \\ 1 \end{bmatrix}, \quad \mathbf{u}_{2} = \begin{bmatrix} 1 \\ -0.5 \end{bmatrix}$$

$$N_1(t) = \mathbf{u}_1^T \mathbf{F}(t) = \begin{bmatrix} 1 \\ 1 \end{bmatrix}^T \begin{bmatrix} 0 \\ F_2(t) \end{bmatrix} = F_2(t)$$
$$N_2(t) = \mathbf{u}_2^T \mathbf{F}(t) = \begin{bmatrix} 1 \\ -0.5 \end{bmatrix}^T \begin{bmatrix} 0 \\ F_2(t) \end{bmatrix} = -0.5F_2(t)$$



EXAMPLE:



MODE Advanced Vibrations

Lecture Three

Elements of Analytical DynamicsMode4

MODE

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Newton's laws were formulated for a single particle

- Can be extended to systems of particles.
- The equations of motion are expressed in terms of physical coordinates vector and force vector.
 - For this reason, Newtonian mechanics is often referred to as vectorial mechanics.

The drawback is that it requires one free-body diagram for each of the masses,

- Necessitating the inclusion of reaction forces and interacting forces.
- These reaction and constraint forces play the role of unknowns, which makes it necessary to work with a surplus of equations of motion, one additional equation for every unknown force.



Analytical mechanics, or *analytical dynamics,* considers the system as a whole:

- Not separate individual components,
- This excludes the reaction and constraint forces automatically.
- This approach, due to Lagrange, permits the formulation of problems of dynamics in terms of:
 - two scalar functions the kinetic energy and the potential energy, and
 - >an infinitesimal expression, the virtual work performed by the nonconservative forces.

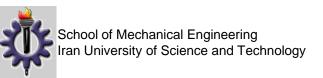


In analytical mechanics the equations of motion are formulated in terms of generalized coordinates and generalized forces:

- > Not necessarily physical coordinates and forces.
- The formulation is independent of any special system of coordinates.
- The development of analytical mechanics required the introduction of the concept of virtual displacements,
 - > led to the development of the calculus of variations.
 - For this reason, analytical mechanics is often referred to as the variational approach to mechanics.

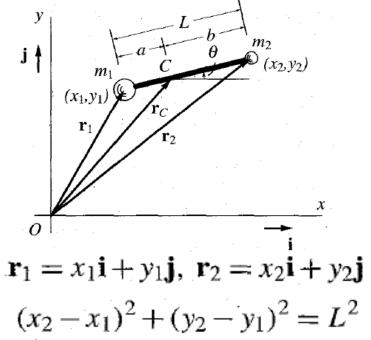


- 6.1 DOF and Generalized Coordinates
- 6.2 The Principle of Virtual Work
- 6.3 The Principle of D'Alembert
- 6.4 The Extended Hamilton's Principle
- 6.5 Lagrange's Equations



6.1 DEGREES OF FREEDOM AND GENERALIZED COORDINATES

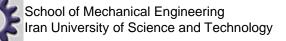
A source of possible difficulties in using Newton's equations is use of physical coordinates, which may not always be independent.



 $\mathbf{r}_{C} = \mathbf{r}_{C}(x_{C}, y_{C}) \text{ and } \theta$ $\mathbf{r}_{1} = \mathbf{r}_{C} - a(\cos\theta\mathbf{i} + \sin\theta\mathbf{j}),$ $\mathbf{r}_{2} = \mathbf{r}_{C} + b(\cos\theta\mathbf{i} + \sin\theta\mathbf{j})$ $a = \frac{m_{2}L}{m_{1} + m_{2}}, \ b = \frac{m_{1}L}{m_{1} + m_{2}}$

Independent coordinates

The generalized coordinates are not unique



6.2 THE PRINCIPLE OF VIRTUAL WORK

The principle of virtual work, due to *Johann Bernoulli*, is basically a statement of the static equilibrium of a mechanical system.

We consider a system of N particles and define the *virtual displacements*, as *infinitesimal changes* in the coordinates.

The virtual displacements must be *consistent with the system constraints,* but are otherwise *arbitrary.*

The virtual displacements, being infinitesimal, obey the rules of differential calculus.



THE PRINCIPLE OF VIRTUAL WORK

$$\mathbf{R}_{i} = \mathbf{F}_{i} + \mathbf{f}_{i} = \mathbf{0}, \ i = 1, 2, \dots, N$$
resultant force on each particle applied force constraint force
$$\overline{\delta W}_{i} = \mathbf{R}_{i} \cdot \delta \mathbf{r}_{i} = 0, \ i = 1, 2, \dots, N$$

$$\overline{\delta W} = \sum_{i=1}^{N} \mathbf{R}_{i} \cdot \delta \mathbf{r}_{i} = 0$$

$$\overline{\delta W} = \sum_{i=1}^{N} \mathbf{F}_{i} \cdot \delta \mathbf{r}_{i} + \sum_{i=1}^{N} \mathbf{f}_{i} \cdot \delta \mathbf{r}_{i} = 0$$
The virtual work performed by
the constraint forces is zero
$$\overline{\delta W} = \sum_{i=1}^{N} \mathbf{F}_{i} \cdot \delta \mathbf{r}_{i} = 0$$

THE PRINCIPLE OF VIRTUAL WORK

When \mathbf{I}_{i} are independent,

$$\overline{\delta W} = \sum_{i=1}^{N} \mathbf{F}_i \cdot \delta \mathbf{r}_i = 0 \quad \longrightarrow \quad \mathbf{F}_i = \mathbf{0}, \ i = 1, 2, \dots, N$$

If not to switch to a set of generalized coordinates: $\mathbf{r}_i = \mathbf{r}_i(q_1, q_2, \dots, q_n), i = 1, 2, \dots, N$

$$\delta \mathbf{r}_{i} = \frac{\partial \mathbf{r}_{i}}{\partial q_{1}} \delta q_{1} + \frac{\partial \mathbf{r}_{i}}{\partial q_{2}} \delta q_{2} + \ldots + \frac{\partial \mathbf{r}_{i}}{\partial q_{n}} \delta q_{n} = \sum_{k=1}^{n} \frac{\partial \mathbf{r}_{i}}{\partial q_{k}} \delta q_{k}, \ i = 1, 2, \ldots, N$$

$$\overline{\delta W} = \sum_{i=1}^{N} \mathbf{F}_{i} \cdot \delta \mathbf{r}_{i} = \sum_{i=1}^{N} \mathbf{F}_{i} \cdot \sum_{k=1}^{n} \frac{\partial \mathbf{r}_{i}}{\partial q_{k}} \delta q_{k} = \sum_{k=1}^{n} \left(\sum_{i=1}^{N} \mathbf{F}_{i} \cdot \frac{\partial \mathbf{r}_{i}}{\partial q_{k}} \right) \delta q_{k} = \sum_{k=1}^{n} Q_{k} \delta q_{k} = 0$$

$$\longrightarrow \qquad Q_{k} = 0, \ k = 1, 2, \ldots, n$$

Generalized forces



THE PRINCIPLE OF D'ALEMBERT

The virtual work principle can be extended to dynamics, in which form it is known as d'Alembert's principle.

$$\mathbf{F}_{i} + \mathbf{f}_{i} - m_{i} \ddot{\mathbf{r}}_{i} = \mathbf{0}, \ i = 1, 2, \dots, N$$
$$(\mathbf{F}_{i} + \mathbf{f}_{i} - m_{i} \ddot{\mathbf{r}}_{i}) \cdot \delta \mathbf{r}_{i} = 0, \ i = 1, 2, \dots, N$$
$$\sum_{i=1}^{N} (\mathbf{F}_{i} - m_{i} \ddot{\mathbf{r}}_{i}) \cdot \delta \mathbf{r}_{i} = 0$$

Lagrange version of d'Alembertls principle



i=1

 $\sum_{i=1}^{N} \mathbf{F}_{i} \cdot \delta \mathbf{r}_{i} = \overline{\delta W}$ The virtual work of all the applied forces,

 $\frac{d}{dt}(m_i \dot{\mathbf{r}}_i \cdot \delta \mathbf{r}) = m_i \ddot{\mathbf{r}}_i \cdot \delta \mathbf{r}_i + m_i \dot{\mathbf{r}}_i \cdot \delta \dot{\mathbf{r}}_i = m_i \ddot{\mathbf{r}}_i \cdot \delta \mathbf{r}_i + \delta (\frac{1}{2}m_i \dot{\mathbf{r}}_i \cdot \dot{\mathbf{r}}_i)$ $= m_i \ddot{\mathbf{r}}_i \cdot \delta \mathbf{r}_i + \delta T_i \longleftarrow \text{The kinetic energy of particle } m_i$

 $\sum (\mathbf{F}_i - m_i \ddot{\mathbf{r}}_i) \cdot \delta \mathbf{r}_i = 0$

$$-\int_{t_1}^{t_2} m_i \ddot{\mathbf{r}}_i \cdot \delta \mathbf{r}_i dt = \int_{t_1}^{t_2} \delta T_i dt - \int_{t_1}^{t_2} \frac{d}{dt} (m_i \dot{\mathbf{r}}_i \cdot \delta \mathbf{r}_i) dt$$
$$= \int_{t_1}^{t_2} \delta T_i dt - m_i \dot{\mathbf{r}}_i \cdot \delta \mathbf{r}_i \Big|_{t_1}^{t_2}$$

It is convenient to choose $\delta \mathbf{r}_i = \mathbf{0}$ at $t = t_1$ and $t = t_2$, $-\int_{t_1}^{t_2} m_i \ddot{\mathbf{r}}_i \cdot \delta \mathbf{r}_i dt = \int_{t_1}^{t_2} \delta T_i dt$, $\delta \mathbf{r}_i = \mathbf{0}$, $t = t_1, t_2$; i = 1, 2, ..., N $-\int_{t_1}^{t_2} \sum_{i=1}^{N} m_i \ddot{\mathbf{r}}_i \cdot \delta \mathbf{r}_i dt = \int_{t_1}^{t_2} \delta T dt$, $\delta \mathbf{r}_i = \mathbf{0}$, i = 1, 2, ..., N; $t = t_1, t_2$

$$\int_{t_1}^{t_2} (\delta T + \overline{\delta W}) dt = 0, \ \delta \mathbf{r}_i = \mathbf{0}, \ i = 1, 2, \dots, N; \ t = t_1, t_2$$

Extended Hamilton's principle



$$\int_{t_1}^{t_2} (\delta T + \overline{\delta W}) dt = 0, \ \delta \mathbf{r}_i = \mathbf{0}, \ i = 1, 2, \dots, N; \ t = t_1, t_2$$

$$\overline{\delta W} = \overline{\delta W_c} + \overline{\delta W}_{nc} = -\delta V + \overline{\delta W}_{nc}$$

where V is the potential energy

$$\int_{t_1}^{t_2} (\delta T - \delta V + \overline{\delta W}_{nc}) dt = 0, \ \delta \mathbf{r}_i = \mathbf{0}, \ i = 1, 2, \dots, N; \ t = t_1, t_2$$

Or in terms of the independent generalized coordinates

$$\int_{t_1}^{t_2} (\delta T - \delta V + \overline{\delta W}_{nc}) dt = 0, \ \delta q_k = 0, \ k = 1, 2, \dots, n; \ t = t_1, t_2$$



A Note:

$$\overline{dW} = \mathbf{F} \cdot d\mathbf{r}$$

$$\overline{dW} = m\mathbf{\ddot{r}} \cdot \mathbf{\dot{r}} dt = m\frac{d\mathbf{\dot{r}}}{dt} \cdot \mathbf{\dot{r}} dt = m\mathbf{\dot{r}} \cdot d\mathbf{\dot{r}} = d\left(\frac{1}{2}m\mathbf{\dot{r}} \cdot \mathbf{\dot{r}}\right)$$

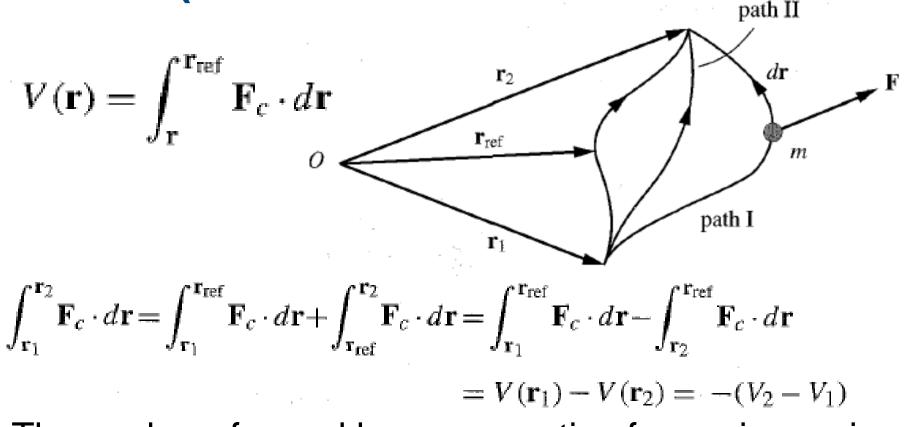
$$T = \frac{1}{2}m\mathbf{\dot{r}} \cdot \mathbf{\dot{r}} \quad \overline{dW} = dT$$

$$\int_{\mathbf{r}_{1}}^{\mathbf{r}_{2}} \mathbf{F} \cdot d\mathbf{r} = \int_{T_{1}}^{T_{2}} dT = T_{2} - T_{1}$$

The work performed by the force F in moving the particle m from position \mathbf{r}_1 to position \mathbf{r}_2 is responsible for a change in the kinetic energy from T_1 to T_2 .



A Note (continued):



The work performed by conservative forces in moving a particle from r_1 to r_2 is equal to the negative of the change in the potential energy from V_1 to V_2



A Note (continued):

$$\int_{\mathbf{r}_{1}}^{\mathbf{r}_{2}} \mathbf{F} \cdot d\mathbf{r} = \int_{\mathbf{r}_{1}}^{\mathbf{r}_{2}} \mathbf{F}_{c} \cdot d\mathbf{r} + \int_{\mathbf{r}_{1}}^{\mathbf{r}_{2}} \mathbf{F}_{nc} \cdot d\mathbf{r}$$

$$T_{2} - T_{1} = -(V_{2} - V_{1}) + \int_{\mathbf{r}_{1}}^{\mathbf{r}_{2}} \mathbf{F}_{nc} \cdot d\mathbf{r}$$

$$E = T + V \qquad \int_{\mathbf{r}_{1}}^{\mathbf{r}_{2}} \mathbf{F}_{nc} \cdot d\mathbf{r} = E_{2} - E_{1}$$

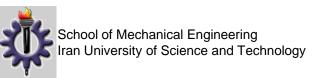
$$\mathbf{F}_{nc} \cdot d\mathbf{r} = dE$$

$$\mathbf{F}_{nc} \cdot \dot{\mathbf{r}} = \dot{E}$$



6 Elements of Analytical Dynamics

- 6.1 DOF and Generalized Coordinates
- 6.2 The Principle of Virtual Work
- 6.3 The Principle of D'Alembert
- 6.4 The Extended Hamilton's Principle
- 6.5 Lagrange's Equations



Advanced Vibrations

Lecture Four: LAGRANGE'S EQUATIONS MODE 4

MODE:

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$$\int_{t_1}^{t_2} (\delta T - \delta V + \overline{\delta W}_{nc}) dt = 0,$$

$$\int_{t_1}^{t_2} \delta L \, dt = 0, \ \delta q_k = 0, \qquad L = T - V$$

Hamilton's principle Lagrangian



Use the extended Hamilton's principle to derive the equations of motion for the two-degree-of-freedom system.

$$T = T_{tr} + T_{rot}$$

$$= \frac{1}{2}m \left[L_{1}^{2}\dot{\theta}_{1}^{2} + L_{1}L_{2}\dot{\theta}_{1}\dot{\theta}_{2}\cos(\theta_{2} - \theta_{1}) + \frac{L_{2}^{2}}{4}\dot{\theta}_{2}^{2} \right] + \frac{1}{2}\frac{mL_{2}^{2}}{12}\dot{\theta}_{2}^{2}$$

$$= \frac{1}{2}m \left[L_{1}^{2}\dot{\theta}_{1}^{2} + L_{1}L_{2}\dot{\theta}_{1}\dot{\theta}_{2}\cos(\theta_{2} - \theta_{1}) + \frac{L_{2}^{2}}{3}\dot{\theta}_{2}^{2} \right]$$

$$V = \int_{\mathbf{r}_{c}}^{\mathbf{r}_{Cref}} (-mg\mathbf{j}) \cdot d\mathbf{r}_{C} = -mg\mathbf{j} \cdot \mathbf{r}_{c} \Big|_{\mathbf{r}_{c}}^{\mathbf{r}_{Cref}}$$

$$= mg \left[L_{1}(1 - \cos\theta_{1}) + \frac{L_{2}}{2}(1 - \cos\theta_{2}) \right] = mg\Delta h$$

$$\overline{\delta W}_{nc} = \mathbf{F} \cdot \delta \mathbf{r}_{B} = F\mathbf{i} \cdot \delta \left[(L_{1}\sin\theta_{1} + L_{2}\sin\theta_{2})\mathbf{i} - (L_{1}\cos\theta_{1} + L_{2}\cos\theta_{2})\mathbf{j} \right]$$

$$= F(L_{1}\cos\theta_{1}\delta\theta_{1} + L_{2}\cos\theta_{2}\delta\theta_{2}) = \Theta_{1}\delta\theta_{1} + \Theta_{2}\delta\theta_{2} = Q_{1}\delta q_{1} + Q_{2}\delta q_{2}$$

$$Q_{1} = \Theta_{1} = FL_{1}\cos\theta_{1}, \ Q_{2} = \Theta_{2} = FL_{2}\cos\theta_{2}$$

$$\begin{split} \delta T = mL_1^2 \dot{\theta}_1 \delta \dot{\theta}_1 + \frac{mL_1L_2}{2} [\dot{\theta}_2 \cos(\theta_2 - \theta_1) \delta \dot{\theta}_1 + \dot{\theta}_1 \cos(\theta_2 - \theta_1) \delta \dot{\theta}_2 \\ &- \dot{\theta}_1 \dot{\theta}_2 \sin(\theta_2 - \theta_1) \delta (\theta_2 - \theta_1)] + \frac{mL_2^2}{3} \dot{\theta}_2 \delta \dot{\theta}_2 \\ = \frac{mL_1L_2}{2} \dot{\theta}_1 \dot{\theta}_2 \sin(\theta_2 - \theta_1) \delta \theta_1 - \frac{mL_1L_2}{2} \dot{\theta}_1 \dot{\theta}_2 \sin(\theta_2 - \theta_1) \delta \theta_2 \\ &+ mL_1 \bigg[L_1 \dot{\theta}_1 + \frac{L_2}{2} \dot{\theta}_2 \cos(\theta_2 - \theta_1) \bigg] \delta \dot{\theta}_1 + mL_2 \bigg[\frac{L_1}{2} \dot{\theta}_1 \cos(\theta_1 - \theta_1) + \frac{L_2}{3} \dot{\theta}_2 \bigg] \delta \dot{\theta}_2 \end{split}$$

$$\delta V = mg\left(L_1\sin\theta_1\delta\theta_1 + \frac{L_2}{2}\sin\theta_2\delta\theta_2\right)$$



$$\begin{split} &\int_{t_1}^{t_2} (\delta T - \delta V + \delta W_{nc}) dt = \int_{t_1}^{t_2} \left\{ \left[\frac{mL_1L_2}{2} \dot{\theta}_1 \dot{\theta}_2 \sin(\theta_2 - \theta_1) - mgL_1 \sin\theta_1 \right. \\ &+ FL_1 \cos\theta_1 \left] \delta \theta_1 + \left[-\frac{mL_1L_2}{2} \dot{\theta}_1 \dot{\theta}_2 \sin(\theta_2 - \theta_1) - \frac{mgL_2}{2} \sin\theta_2 + FL_2 \cos\theta_2 \right] \delta \theta_2 \right. \\ &+ mL_1 \left[L_1 \dot{\theta}_1 + \frac{L_2}{2} \dot{\theta}_2 \cos(\theta_2 - \theta_1) \right] \delta \dot{\theta}_1 + mL_2 \left[\frac{L_1}{2} \dot{\theta}_1 \cos(\theta_2 - \theta_1) + \frac{L_2}{3} \dot{\theta}_2 \right] \delta \dot{\theta}_2 \right\} dt = 0 \end{split}$$

Only the virtual displacements are arbitrary.



$$\int_{t_1}^{t_2} m L_1 \left[L_1 \dot{\theta}_1 + \frac{L_2}{2} \dot{\theta}_2 \cos(\theta_2 - \theta_1) \right] \delta \dot{\theta}_1 dt = m L_1 \left[L_1 \dot{\theta}_1 + \frac{L_2}{2} \dot{\theta}_2 \cos(\theta_2 - \theta_1) \right] \delta \theta_1 \Big|_{t_1}^{t_2}$$

$$-\int_{t_1}^{t_2} mL_1 \frac{d}{dt} \left[L_1 \dot{\theta}_1 + \frac{L_2}{2} \dot{\theta}_2 \cos(\theta_2 - \theta_1) \right] \delta\theta_1 dt$$

= $-\int_{t_1}^{t_2} mL_1 \left[L_1 \ddot{\theta}_1 + \frac{L_2}{2} \ddot{\theta}_2 \cos(\theta_2 - \theta_1) - \frac{L_2}{2} \dot{\theta}_2 (\dot{\theta}_2 - \dot{\theta}_1) \sin(\theta_2 - \theta_1) \right] \delta\theta_1 dt$

$$\int_{t_1}^{t_2} mL_2 \left[\frac{L_1}{2} \dot{\theta}_1 \cos(\theta_2 - \theta_1) + \frac{L_2}{3} \dot{\theta}_2 \right] \delta \dot{\theta}_2 dt = mL_2 \left[\frac{L_1}{2} \dot{\theta}_1 \cos(\theta_2 - \theta_1) + \frac{L_2}{3} \dot{\theta}_2 \right] \delta \theta_2 \Big|_{t_1}^{t_2}$$

$$-\int_{t_1}^{t_2} mL_2 \frac{d}{dt} \left[\frac{L_1}{2} \dot{\theta}_1 \cos(\theta_2 - \theta_1) + \frac{L_2}{3} \dot{\theta}_2 \right] \delta\theta_2 dt$$

= $-\int_{t_1}^{t_2} mL_2 \left[\frac{L_1}{2} \ddot{\theta}_1 \cos(\theta_2 - \theta_1) - \frac{L_1}{2} \dot{\theta}_1 (\dot{\theta}_2 - \dot{\theta}_1) \sin(\theta_2 - \theta_1) + \frac{L_2}{3} \ddot{\theta}_2 \right] \delta\theta_2 dt$



$$\begin{split} \delta\theta_1 &= \delta\theta_2 = 0 \text{ at } t = t_1, \ t_2. \\ \int_{t_1}^{t_2} \left\{ - \left[mL_1^2 \ddot{\theta}_1 + \frac{mL_1L_2}{2} \ddot{\theta}_2 \cos(\theta_2 - \theta_1) - \frac{mL_1L_2}{2} \dot{\theta}_2^2 \sin(\theta_2 - \theta_1) \right. \\ &+ mgL_1 \sin\theta_1 - FL_1 \cos\theta_1 \left] \delta\theta_1 - \left[\frac{mL_1L_2}{2} \ddot{\theta}_1 \cos(\theta_2 - \theta_1) + \frac{mL_2^2}{3} \ddot{\theta}_2 \right. \\ &+ \frac{mL_1L_2}{2} \dot{\theta}_1^2 \sin(\theta_2 - \theta_1) + \frac{mgL_2}{2} \sin\theta_2 - FL_2 \cos\theta_2 \left] \delta\theta_2 \right\} dt = 0 \\ mL_1^2 \ddot{\theta}_1 + \frac{mL_1L_2}{2} \left[\ddot{\theta}_2 \cos(\theta_2 - \theta_1) - \dot{\theta}_2^2 \sin(\theta_2 - \theta_1) \right] + mgL_1 \sin\theta_1 = FL_1 \cos\theta_1 \\ \frac{mL_1L_2}{2} \left[\ddot{\theta}_1 \cos(\theta_2 - \theta_1) + \dot{\theta}_1^2 \sin(\theta_2 - \theta_1) \right] + \frac{mL_2^2}{3} \ddot{\theta}_2 + \frac{mgL_2}{2} \sin\theta_2 = FL_2 \cos\theta_2 \end{split}$$

For many problems the extended Hamilton's principle is not the most efficient method for deriving equations of motion:

Involves routine operations that must be carried out every time the principle is applied,

The integrations by parts.

The extended Hamilton's principle is used to generate a more expeditious method for deriving equations of motion, *Lagrange's equations*.



$$T = T(q_1, q_2, \dots, q_n, \dot{q}_1, \dot{q}_2, \dots, \dot{q}_n)$$
$$\delta T = \sum_{k=1}^n \left(\frac{\partial T}{\partial q_k} \delta q_k + \frac{\partial T}{\partial \dot{q}_k} \delta \dot{q}_k \right)$$
$$V = V(q_1, q_2, \dots, q_n) \quad \delta V = \sum_{k=1}^n \frac{\partial V}{\partial q_k} \delta q_k$$
$$\overline{\delta W}_{nc} = \sum_{k=1}^n Q_k \delta q_k$$

$$\int_{t_1}^{t_2} (\delta T - \delta V + \overline{\delta W}_{nc}) dt = \int_{t_1}^{t_2} \sum_{k=1}^{n} \left[\left(\frac{\partial T}{\partial q_k} - \frac{\partial V}{\partial q_k} + Q_k \right) \delta q_k + \frac{\partial T}{\partial \dot{q}_k} \delta \dot{q}_k \right] dt = 0,$$

$$\delta q_k = 0, \ k = 1, 2, \dots, n; \ t = t_1, t_2$$



 $\int_{t_1}^{t_2} \frac{\partial T}{\partial \dot{q}_k} \delta \dot{q}_k dt = \int_{t_1}^{t_2} \frac{\partial T}{\partial \dot{q}_k} \frac{d}{dt} \delta q_k dt = \frac{\partial T}{\partial \dot{q}_k} \delta q_k \Big|_{t_1}^{t_2} - \int_{t_1}^{t_2} \frac{d}{dt} \left(\frac{\partial t}{\partial \dot{q}_k}\right) \delta q_k dt$ $\delta q_k = 0, \ k = 1, 2, \dots, n; \ t = t_1, t_2 \implies = -\int_{t_1}^{t_2} \frac{d}{dt} \left(\frac{\partial T}{\partial \dot{q}_k}\right) \delta q_k dt, \ k = 1, 2, \dots, n$

$$\int_{t_1}^{t_2} \sum_{k=1}^{n} \left[\frac{\partial T}{\partial q_k} - \frac{\partial V}{\partial q_k} + Q_k - \frac{d}{dt} \left(\frac{\partial T}{\partial \dot{q}_k} \right) \right] \delta q_k \, dt = 0$$
$$\frac{d}{dt} \left(\frac{\partial T}{\partial t} \right) - \frac{\partial T}{\partial t} + \frac{\partial V}{\partial t} = 0, \quad k = 1, 2, \dots, n$$

$$\overline{dt}\left(\frac{\partial \dot{q}_k}{\partial \dot{q}_k}\right) = \overline{\partial q_k} + \overline{\partial q_k} = \mathcal{Q}_k, \ \kappa = 1, 2, \dots, n$$



Derive Lagrange's equations of motion for the system

$$\frac{d}{dt} \left(\frac{\partial T}{\partial \dot{\theta}_k} \right) - \frac{\partial T}{\partial \theta_k} + \frac{\partial V}{\partial \theta_k} = \Theta_k, \ k = 1, 2$$

$$V = mg \left[L_1(1 - \cos\theta_1) + \frac{L_2}{2}(1 - \cos\theta_2) \right]$$

$$\frac{\partial V}{\partial \theta_1} = mg L_1 \sin\theta_1, \ \frac{\partial V}{\partial \theta_2} = \frac{mg L_2}{2} \sin\theta_2$$

$$\overline{\delta W}_{nc} = FL_1 \cos\theta_1 \delta\theta_1 + FL_2 \cos\theta_2 \delta\theta_2$$

$$\Theta_1 = FL_1 \cos\theta_1, \ \Theta_2 = FL_2 \cos\theta_2$$

$$T = \frac{1}{2}m \left[L_1^2 \dot{\theta}_1^2 + L_1 L_2 \dot{\theta}_1 \dot{\theta}_2 \cos(\theta_2 - \theta_1) + \frac{L_2^2}{3} \dot{\theta}_2^2 \right]$$

$$\frac{\partial T}{\partial \dot{\theta}_1} = mL_1^2 \dot{\theta}_1 + \frac{mL_1L_2}{2} \dot{\theta}_2 \cos(\theta_2 - \theta_1)$$

$$\frac{\partial T}{\partial \dot{\theta}_2} = \frac{mL_1L_2}{2} \dot{\theta}_1 \cos(\theta_2 - \theta_1) + \frac{mL_2^2}{3} \dot{\theta}_2$$

$$\frac{d}{dt}\left(\frac{\partial T}{\partial \dot{\theta}_1}\right) = mL_1^2\ddot{\theta}_1 + \frac{mL_1l_2}{2}\left[\ddot{\theta}_2\cos(\theta_2 - \theta_1) - \dot{\theta}_2(\dot{\theta}_2 - \dot{\theta}_1)\sin(\theta_2 - \theta_1)\right]$$
$$\frac{d}{dt}\left(\frac{\partial T}{\partial \dot{\theta}_2}\right) = \frac{mL_1L_2}{2}\left[\ddot{\theta}_1\cos(\theta_2 - \theta_1) - \dot{\theta}_1(\dot{\theta}_2 - \dot{\theta}_1)\sin(\theta_2 - \theta_1)\right] + \frac{mL_2^2}{3}\ddot{\theta}_2$$



$$T = \frac{1}{2}m \left[L_1^2 \dot{\theta}_1^2 + L_1 L_2 \dot{\theta}_1 \dot{\theta}_2 \cos(\theta_2 - \theta_1) + \frac{L_2^2}{3} \dot{\theta}_2^2 \right]$$

$$\frac{\partial T}{\partial \theta_1} = \frac{mL_1L_2}{2}\dot{\theta}_1\dot{\theta}_2\sin(\theta_2-\theta_1), \ \frac{\partial T}{\partial \theta_2} = -\frac{mL_1L_2}{2}\dot{\theta}_1\dot{\theta}_2\sin(\theta_2-\theta_1)$$

$$\begin{split} mL_{1}^{2}\ddot{\theta}_{1} + \frac{mL_{1}L_{2}}{2}[\ddot{\theta}_{2}\cos(\theta_{2}-\theta_{1}) - \dot{\theta}_{2}^{2}\sin(\theta_{2}-\theta_{1})] + mgL_{1}\sin\theta_{1} = FL_{1}\cos\theta_{1} \\ \frac{mL_{1}L_{2}}{2}[\ddot{\theta}_{1}\cos(\theta_{2}-\theta_{1}) + \dot{\theta}_{1}^{2}\sin(\theta_{2}-\theta_{1})] + \frac{mL_{2}^{2}}{3}\ddot{\theta}_{2} + \frac{mgL_{2}}{2}\sin\theta_{2} = FL_{2}\cos\theta_{2} \end{split}$$



- Lagrange's equations are more efficient, the extended Hamilton principle is more versatile.
- In fact, it can produce results in cases in which Lagrange's equations cannot, most notably in the case of distributed-parameter systems.



Advanced Vibrations

Lecture Five: MULTI-DEGREE-OF-FREEDOMDE 4 SYSTEMS

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7.Multi-Degree-of-Freedom Systems

7.1 Equations of Motion for Linear Systems

7.2 Flexibility and Stiffness Influence Coefficients

7.3 Properties of the Stiffness and Mass Coefficients

7.4 Lagrange's Equations Linearized about Equilibrium

7.5 Linear Transformations : Coupling

7.6 Undamped Free Vibration :The Eigenvalue Problem

7.7 Orthogonality of Modal Vectors

7.8 Systems Admitting Rigid-Body Motions

7.9 Decomposition of the Response in Terms of Modal Vectors

7.10 Response to Initial Excitations by Modal Analysis

7.11 Eigenvalue Problem in Terms of a Single Symmetric Matrix

7.12 Geometric Interpretation of the Eigenvalue Problem

7.13 Rayleigh's Quotient and Its Properties

7.14 Response to Harmonic External Excitations

7.15 Response to External Excitations by Modal Analysis

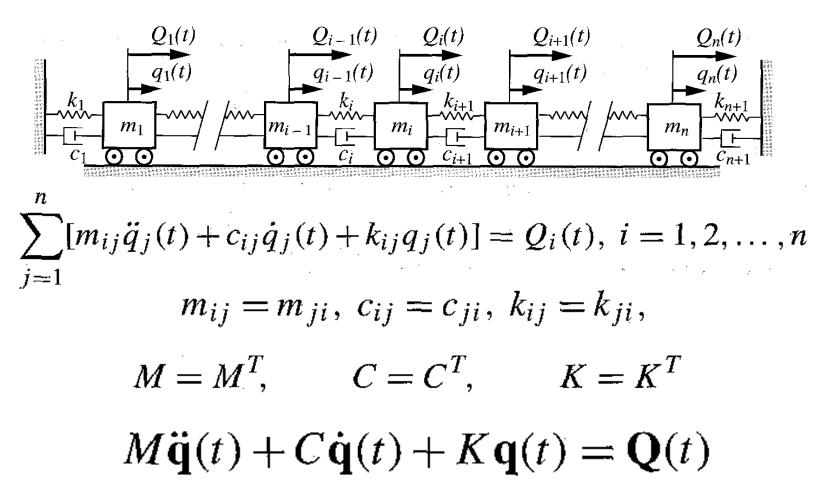
- 7.15.1 Undamped systems
- 7.15.2 Systems with proportional damping

7.16 Systems with Arbitrary Viscous Damping

7.17 Discrete-Time Systems



7.1 EQUATIONS OF MOTION FOR LINEAR SYSTEMS



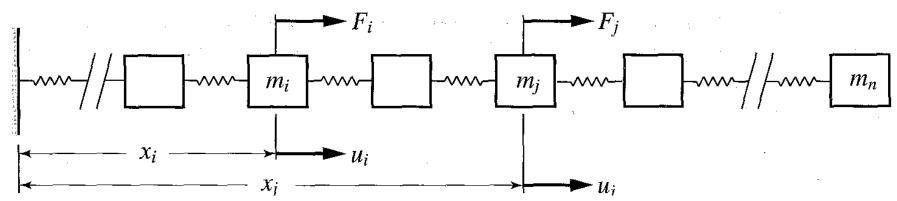


The stiffness coefficients can be obtained by other means, not necessarily involving the equations of motion.

The stiffness coefficients are more properly known as stiffness influence coefficients, and can be derived by using its definition.

There is one more type of influence coefficients, namely, *flexibility influence coefficients.*

They are intimately related to the stiffness influence coefficients.



We define the flexibility influence coefficient a_{ij} as the displacement of point x_i , due to a unit force, $F_j = 1$. $u_i = \sum_{i=1}^n a_{ij} F_j$



The stiffness influence coefficient \mathbf{k}_{ij} is the force required at \mathbf{x}_i to produce a unit displacement at point \mathbf{x}_j , and displacements at all other points are zero.

To obtain zero displacements at all points the forces must simply hold these points fixed.

$$F_i = \sum_{j=1}^n k_{ij} u_j$$



 $[a_{ij}] = A, [k_{ij}] = K$

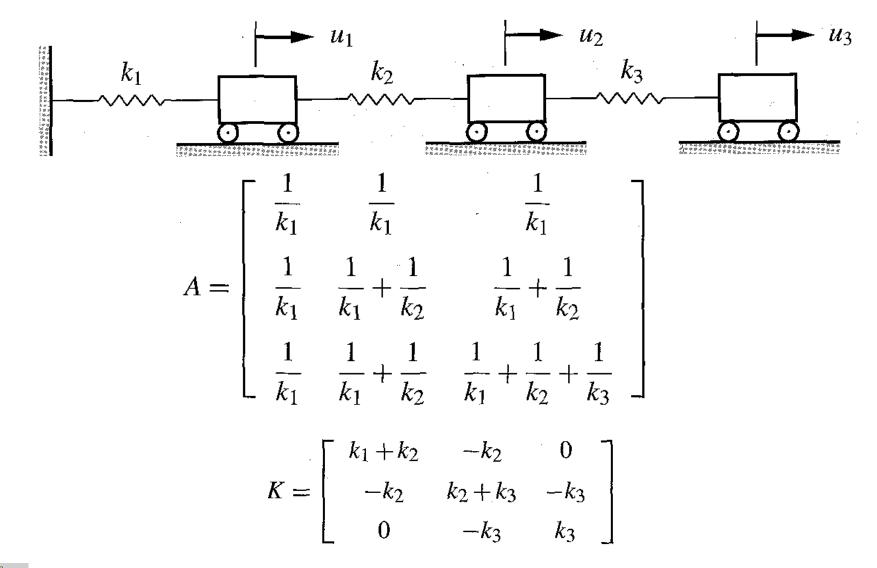
 $\mathbf{u} = A\mathbf{F}$ $\mathbf{F} = K\mathbf{u}$

 $\mathbf{u} = A\mathbf{F} = AK\mathbf{u}$

 $A = K^{-1}, K = A^{-1}$







7.3 PROPERTIES OF THE STIFFNESS AND MASS COEFFICIENTS

The potential energy of a single linear spring:

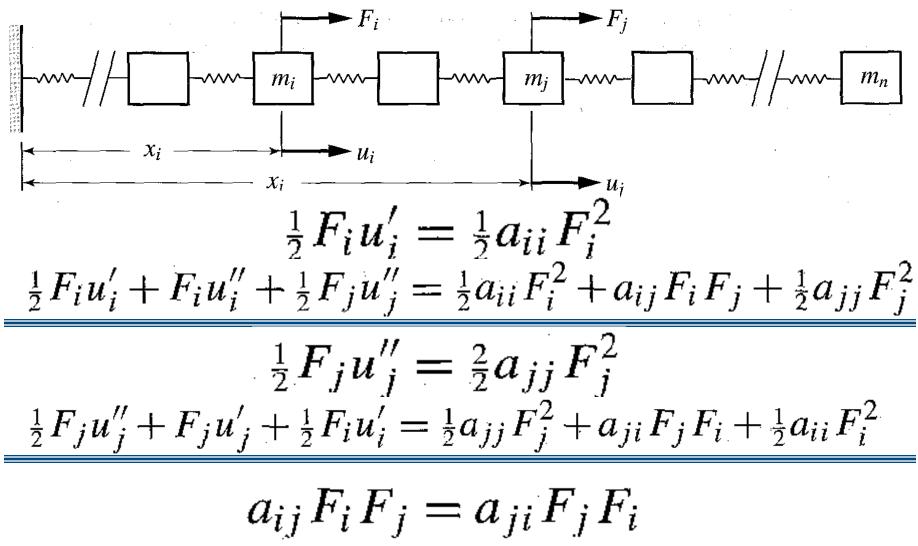
$$V = \int_{u}^{0} F_{\zeta} d\zeta = \int_{u}^{0} (-k\zeta) d\zeta = \frac{1}{2} k u^{2} = \frac{1}{2} F u$$

By analogy the elastic potential energy for a system is: $V = \sum_{i=1}^{n} V_i = \frac{1}{2} \sum_{i=1}^{n} F_{i} u_i$

$$V = \frac{1}{2} \sum_{i=1}^{n} u_i \left(\sum_{j=1}^{n} k_{ij} u_j \right)^{n} = \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{n} k_{ij} u_i u_j$$
$$V = \frac{1}{2} \sum_{i=1}^{n} F_i \left(\sum_{j=1}^{n} a_{ij} F_j \right)^{n} = \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{n} a_{ij} F_i F_j$$



Symmetry Property:



Maxwell's reciprocity theorem:

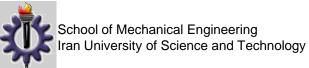
$$a_{ij} = a_{ji} \longrightarrow$$

$$k_{ij} = k_{ji}, \ i, j = 1, \ 2, \dots, n$$

$$A = A^{T}, \ K = K^{T}$$

$$V = \frac{1}{2} \mathbf{u}^{T} K \mathbf{u} \qquad V = \frac{1}{2} \mathbf{F}^{T} A \mathbf{F}$$

$$T = \frac{1}{2} \sum_{i=1}^{n} m_{i} \dot{u}_{i}^{2} \longrightarrow T = \frac{1}{2} \dot{\mathbf{u}}^{T} M \dot{\mathbf{u}}$$



7.4 LAGRANGE'S EQUATIONS LINEARIZED ABOUT EQUILIBRIUM

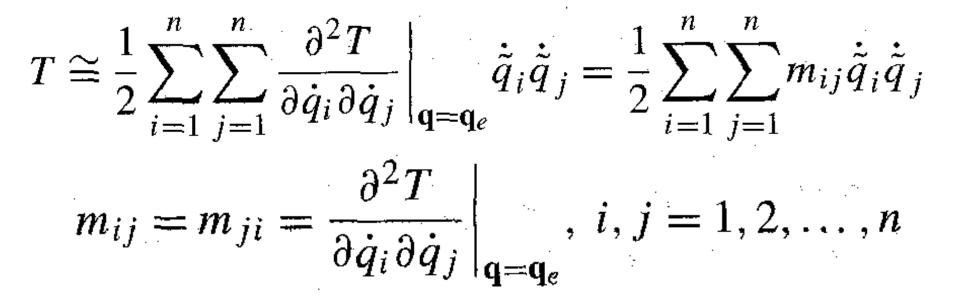
$$\frac{d}{dt} \left(\frac{\partial T}{\partial \dot{q}_k} \right) - \frac{\partial T}{\partial q_k} + \frac{\partial V}{\partial q_k} = Q_k, \ k = 1, 2, \dots, n$$

$$T = T(q_1, q_2, \dots, q_n, \ \dot{q}_1, \dot{q}_2, \dots, \dot{q}_n) \ V = V(q_1, q_2, \dots, q_n)$$

$$\overline{\delta W}_{nc} = \sum_{k=1}^n Q_k \delta q_k \qquad Q_{k \text{visc}} = -\frac{\partial \mathcal{F}}{\partial q_k}, \ k = 1, 2, \dots, n$$
Rayleigh's dissipation function
$$\frac{d}{dt} \left(\frac{\partial T}{\partial \dot{q}_k} \right) - \frac{\partial T}{\partial q_k} + \frac{\partial V}{\partial q_k} + \frac{\partial \mathcal{F}}{\partial \dot{q}_k} = Q_k, \ k = 1, 2, \dots, n$$

7.4 LAGRANGE'S EQUATIONS LINEARIZED ABOUT EQUILIBRIUM

$$q_k(t) = q_{ek} + \tilde{q}_k(t), \ k = 1, 2, \dots, n$$
$$\dot{q}_k(t) = \dot{\tilde{q}}_k(t), \ k = 1, 2, \dots, n$$



7.4 LAGRANGE'S EQUATIONS LINEARIZED ABOUT EQUILIBRIUM

$$V \cong V(\mathbf{q}_{e}) + \sum_{i=1}^{n} \frac{\partial V}{\partial q_{i}} \Big|_{\mathbf{q}=\mathbf{q}_{e}} \tilde{q}_{i} + \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{n} \frac{\partial^{2} V}{\partial q_{i} \partial q_{j}} \Big|_{\mathbf{q}=\mathbf{q}_{e}} \tilde{q}_{i} \tilde{q}_{j}$$
$$= V(\mathbf{q}_{e}) + \sum_{i=1}^{n} \frac{\partial V}{\partial q_{i}} \Big|_{\mathbf{q}=\mathbf{q}_{e}} \tilde{q}_{i} + \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{n} k_{ij} \tilde{q}_{i} \tilde{q}_{j}$$
$$k_{ij} = k_{ji} = \frac{\partial^{2} V}{\partial q_{i} \partial q_{j}} \Big|_{\mathbf{q}=\mathbf{q}_{e}}, \quad i, j = 1, 2, \dots, n$$



7.4 LAGRANGE'S EQUATIONS LINEARIZED ABOUT EQUILIBRIUM

 $\mathcal{F} = \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{n} c_{ij} \dot{\tilde{q}}_{i} \dot{\tilde{q}}_{j}$ n $\sum (m_{ij}\ddot{q}_j + c_{ij}\dot{q}_j + k_{ij}q_j) = Q_i, \ i = 1, 2, \dots, n$ j=1 $M\ddot{\mathbf{q}}(t) + C\dot{\mathbf{q}}(t) + K\mathbf{q} = \mathbf{Q}(t)$ $T = \frac{1}{2} \dot{\mathbf{q}}^T M \dot{\mathbf{q}}$ $\mathcal{F} = \frac{1}{2} \dot{\mathbf{q}}^T C \dot{\mathbf{q}}$ $\overline{\delta W}_{nc} = \mathbf{O}^T \delta \mathbf{q}$ $V = \frac{1}{2} \mathbf{q}^T K \mathbf{q}^T$



7.5 LINEAR TRANSFORMATIONS. COUPLING

 $M\ddot{\mathbf{q}}(t) + K\mathbf{q}(t) = \mathbf{Q}(t)$ $\mathbf{q}(t) = U\boldsymbol{\eta}(t)$ $\dot{\mathbf{q}}(t) = U\dot{\boldsymbol{\eta}}(t), \ \ddot{\mathbf{q}}(t) = U\ddot{\boldsymbol{\eta}}(t)$ $MU\ddot{\eta}(t) + KU\eta(t) = \mathbf{Q}(t)$ $M'\ddot{\eta}(t) + K'\eta(t) = \mathbf{N}(t)$ $M' = U^T M U = M'^T$, $K' = U^T K U = K'^T$ $\mathbf{N}(t) = U^T \mathbf{O}(t)$



Derivation of the matrices *M*' and K' in a more natural manner

$$T = \frac{1}{2} \dot{\mathbf{q}}^T M \dot{\mathbf{q}} \qquad V = \frac{1}{2} \mathbf{q}^T K \mathbf{q}$$
$$\mathbf{q}(t) = U \boldsymbol{\eta}(t)$$
$$T = \frac{1}{2} \dot{\boldsymbol{\eta}}^T(t) M' \dot{\boldsymbol{\eta}}(t), \quad V = \frac{1}{2} \boldsymbol{\eta}^T(t) K' \boldsymbol{\eta}(t)$$
$$M' = U^T M U = M'^T, \quad K' = U^T K U = K'^T$$

 $M'_{jj}\ddot{\eta}_j(t) + K'_{jj}\eta_j(t) = N_j(t) \ j = 1, 2, ..., n$



7.Multi-Degree-of-Freedom Systems

7.1 Equations of Motion for Linear Systems

7.2 Flexibility and Stiffness Influence Coefficients

7.3 Properties of the Stiffness and Mass Coefficients

7.4 Lagrange's Equations Linearized about Equilibrium

7.5 Linear Transformations : Coupling

7.6 Undamped Free Vibration :The Eigenvalue Problem

7.7 Orthogonality of Modal Vectors

7.8 Systems Admitting Rigid-Body Motions

7.9 Decomposition of the Response in Terms of Modal Vectors

7.10 Response to Initial Excitations by Modal Analysis

7.11 Eigenvalue Problem in Terms of a Single Symmetric Matrix

7.12 Geometric Interpretation of the Eigenvalue Problem

7.13 Rayleigh's Quotient and Its Properties

7.14 Response to Harmonic External Excitations

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- 7.15.1 Undamped systems
- 7.15.2 Systems with proportional damping

7.16 Systems with Arbitrary Viscous Damping

7.17 Discrete-Time Systems



Advanced Vibrations

Lecture 6 Multi-Degree-of-Freedom Systems (Ch7)

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MODE

7.Multi-Degree-of-Freedom Systems

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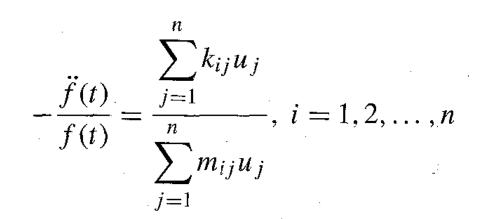
7.6 UNDAMPED FREE VIBRATION. THE EIGENVALUE PROBLEM $M\ddot{\mathbf{q}}(t) + K\mathbf{q}(t) = \mathbf{0}$

 $\sum m_{ij} \ddot{q}_{j}(t) + \sum k_{ij} q_{j}(t) = 0, \ i = 1, 2, \dots, n$ i=1synchronous motion $q_{j}(t) = u_{j}f(t), \ j = 1, 2..., n$ $\ddot{f}(t) \sum m_{ij} u_j + f(t) \sum k_{ij} u_j = 0, \ i = 1, 2, \dots, n$ i=1



7.6 UNDAMPED FREE VIBRATION. THE EIGENVALUE PROBLEM

n



$$\sum_{j=1}^{\infty} (k_{ij} - \lambda m_{ij}) u_j = 0,$$

$$i = 1, 2, \dots, n$$

$$\ddot{f}(t) + \lambda f(t) = 0$$

$$f(t) = Ae^{st}$$
$$s^{2} + \lambda = 0$$
$$\frac{s_{1}}{s_{2}} = \pm \sqrt{-\lambda} \begin{vmatrix} s_{1} \\ s_{2} \end{vmatrix} = \pm i\omega$$

$$f(t) = A_1 e^{i\omega t} + A_2 e^{-i\omega t}$$
$$f(t) = C\cos(\omega t - \phi)$$



7.6 UNDAMPED FREE VIBRATION. THE EIGENVALUE PROBLEM $K\mathbf{u} = \omega^2 M \mathbf{u}$ $\Delta(\omega^2) = \det[K - \omega^2 M] = 0$ characteristic polynomial

$$\omega_1 \leq \omega_2 \leq \cdots \leq \omega_n$$

In general, all frequencies are distinct, except:

➢ In *degenerate* cases,

- They cannot occur in one-dimensional structures;
- They can occur in two-dimensional symmetric structures.

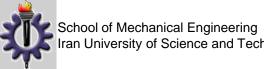
7.6 UNDAMPED FREE VIBRATION. THE EIGENVALUE PROBLEM $K\mathbf{u}_r = \omega_r^2 M \mathbf{u}_r, r = 1, 2, ..., n$

The shape of the natural modes is unique but the amplitude is not.

A very convenient normalization scheme consists of setting:

$$\mathbf{u}_{r}^{T} M \mathbf{u}_{r} = 1, r = 1, 2, ..., n$$

 $\mathbf{u}_{r}^{T} K \mathbf{u}_{r} = \omega_{r}^{2}, r = 1, 2, ..., n$



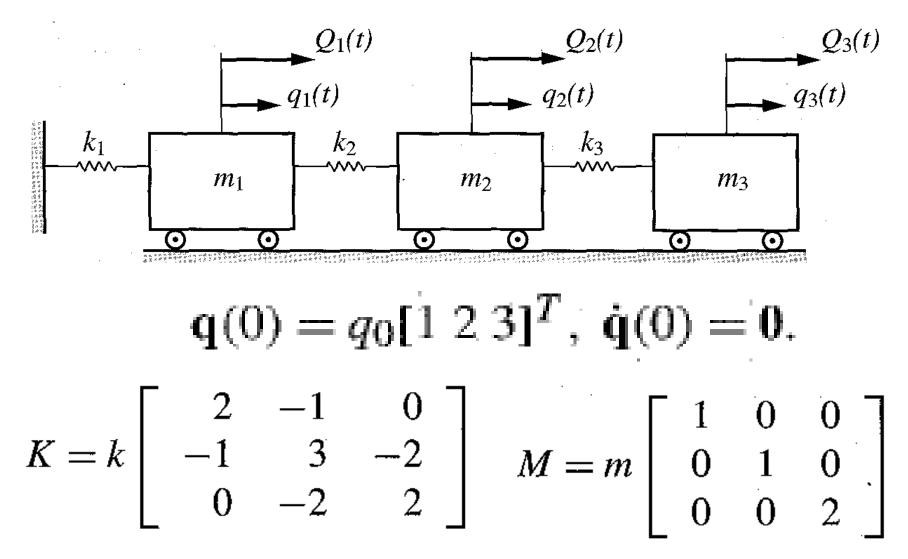
7.6 UNDAMPED FREE VIBRATION. THE EIGENVALUE PROBLEM

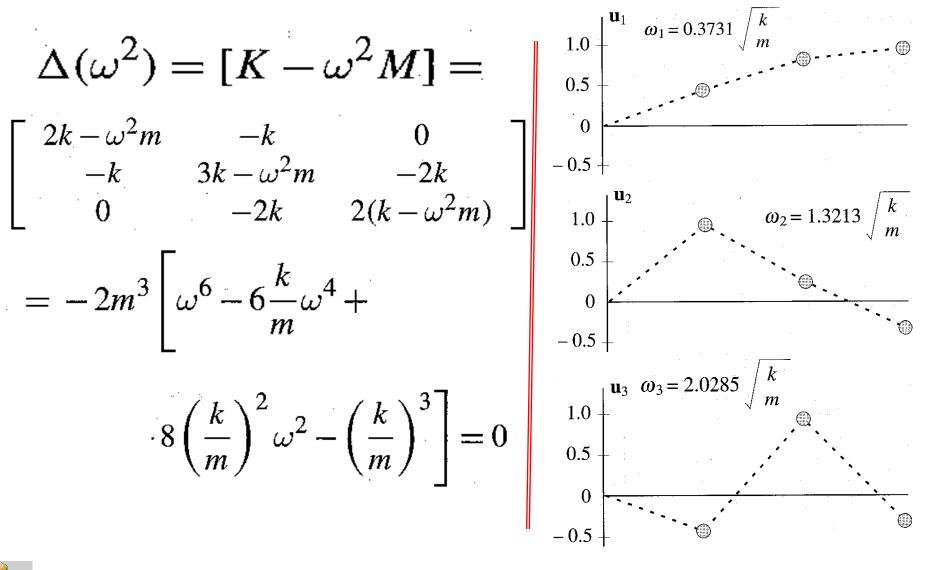
$$\mathbf{q}_r(t) = \mathbf{u}_r f_r(t), \ r = 1, 2, ..., n$$

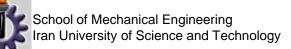
$f_r(t) = C_r \cos(\omega_r t - \phi_r), \ r = 1, 2, \dots, n$

$$\mathbf{q}(t) = \sum_{r=1}^{n} \mathbf{q}_r(t) = \sum_{r=1}^{n} \mathbf{u}_r f_r(t) = U\mathbf{f}(t)$$









$$\mathbf{q}(t) = \sum_{r=1}^{3} \mathbf{u}_{r} f_{r}(t) = \sum_{r=1}^{3} C_{r} \mathbf{u}_{r} \cos(\omega_{r} t - \phi_{r})$$

$$= C_{1} \begin{bmatrix} 0.4626 \\ 0.8608 \\ 1.0000 \end{bmatrix} \cos(0.3731 \sqrt{\frac{k}{m}} t - \phi_{1})$$

$$+ C_{2} \begin{bmatrix} 1.0000 \\ 0.2541 \\ -0.3407 \end{bmatrix} \cos\left(1.3213 \sqrt{\frac{k}{m}} t - \phi_{2}\right)$$

$$+ C_{3} \begin{bmatrix} -0.4728 \\ 1.0000 \\ -0.3210 \end{bmatrix} \cos\left(2.0285 \sqrt{\frac{k}{m}} t - \phi_{3}\right)$$



$$\mathbf{q}(0) = C_1 \begin{bmatrix} 0.4626\\ 0.8608\\ 1.0000 \end{bmatrix} \cos\phi_1 + C_2 \begin{bmatrix} 1.0000\\ 0.2541\\ -0.3407 \end{bmatrix} \cos\phi_2 \\ + C_3 \begin{bmatrix} -0.4728\\ 1.0000\\ -0.3210 \end{bmatrix} \cos\phi_3 = q_0 \begin{bmatrix} 1\\ 2\\ 3 \end{bmatrix}$$

$$0.3731\sqrt{\frac{k}{m}}C_1 \begin{bmatrix} 0.4626\\ 0.8608\\ 1.0000 \end{bmatrix} \sin\phi_1 + 1.3213\sqrt{\frac{k}{m}}C_2 \begin{bmatrix} 1.0000\\ 0.2541\\ -0.3407 \end{bmatrix} \sin\phi_2 \\ + 2.0285\sqrt{\frac{k}{m}}C_3 \begin{bmatrix} -0.4728\\ 1.0000\\ -0.3210 \end{bmatrix} \sin\phi_3 = \begin{bmatrix} 0\\ 0\\ 0 \end{bmatrix}$$

$$\phi_1 = \phi_2 = \phi_3 = 0$$

 $C_1 = 2.7696q_0, \ C_2 = -0.4132q_0, \ C_3 = -0.2791q_0$

$$\mathbf{x}(t) = q_0 \left\{ \begin{bmatrix} 1.2812\\ 2.3841\\ 2.7696 \end{bmatrix} \cos 0.3731 \sqrt{\frac{k}{m}} t + \begin{bmatrix} -0.4132\\ -0.1050\\ 0.1408 \end{bmatrix} \cos 1.3213 \sqrt{\frac{k}{m}} t + \begin{bmatrix} 0.1320\\ -0.2791\\ 0.0896 \end{bmatrix} \cos 2.0285 \sqrt{\frac{k}{m}} t \right\}$$



7.7 ORTHOGONALITY OF MODAL VECTORS

$$K\mathbf{u}_{r} = \omega_{r}^{2}M\mathbf{u}_{r}, \quad K\mathbf{u}_{s} = \omega_{s}^{2}M\mathbf{u}_{s}$$
$$\mathbf{u}_{s}^{T}K\mathbf{u}_{r} = \omega_{r}^{2}\mathbf{u}_{s}^{T}M\mathbf{u}_{r}$$
$$\mathbf{u}_{r}^{T}K\mathbf{u}_{s} = \omega_{s}^{2}\mathbf{u}_{r}^{T}M\mathbf{u}_{s}$$
$$(\omega_{r}^{2} - \omega_{s}^{2})\mathbf{u}_{s}^{T}M\mathbf{u}_{r} = 0$$
$$\mathbf{u}_{s}^{T}M\mathbf{u}_{r} = 0, \quad \mathbf{u}_{s}^{T}K\mathbf{u}_{r} = 0, \quad r \neq s$$



7.7 ORTHOGONALITY OF MODAL VECTORS

$$\mathbf{u}_r^T M \mathbf{u}_s = \delta_{rs}, \ \mathbf{u}_r^T K \mathbf{u}_s = \omega_r^2 \delta_{rs}, \ r, s = 1, 2, \dots, n$$

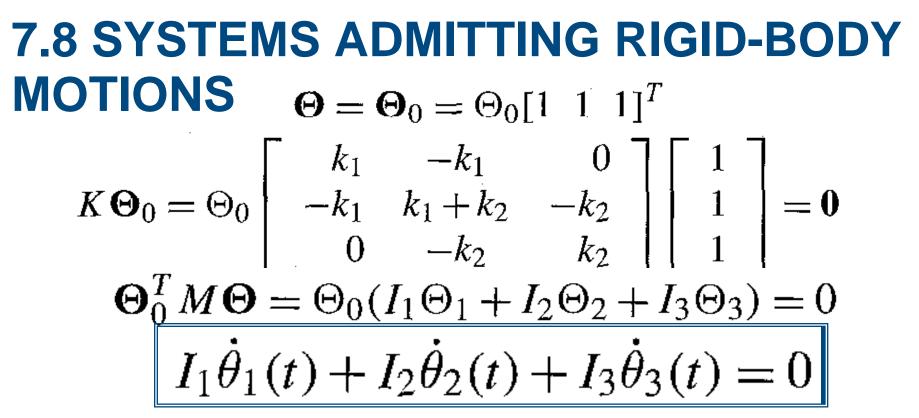
$KU = MU\Omega$

$U^T M U = I, \ U^T K U = \Omega$



7.8 SYSTEMS ADMITTING RIGID-BODY MOTIONS $T = \frac{1}{2}(I_1\dot{\theta}_1^2 + I_2\dot{\theta}_2^2 + I_3\dot{\theta}_3^2) = \frac{1}{2}\dot{\theta}^T M\dot{\theta}$ $V = \frac{1}{2} [k_1 (\theta_2 - \theta_1)^2 + k_2 (\theta_3 - \theta_2)^2] = \frac{1}{2} \theta^T K \theta$ $\theta_1(t)$ $M = \left| \begin{array}{ccc} I_1 & 0 & 0 \\ 0 & I_2 & 0 \\ 0 & 0 & I_2 \end{array} \right|$ $K = \begin{bmatrix} k_1 & -k_1 & 0 \\ -k_1 & k_1 + k_2 & -k_2 \\ 0 & -k_2 & k_2 \end{bmatrix}$ GJ_2





The orthogonality of the rigid-body mode to the elastic modes is equivalent to the preservation of zero angular momentum in pure elastic motion.

$$\theta_3 = -\frac{I_1}{I_3}\theta_1 - \frac{I_2}{I_3}\theta_2$$



7.8 SYSTEMS ADMITTING RIGID-BODY MOTIONS

$$\boldsymbol{\theta} = \begin{bmatrix} \theta_1 & \theta_2 & \theta_3 \end{bmatrix}^T \quad \boldsymbol{\theta}' = \begin{bmatrix} \theta_1 & \theta_2 \end{bmatrix}^T$$
$$\boldsymbol{\theta} = \boldsymbol{C} \boldsymbol{\theta}' \qquad \boldsymbol{C} = \begin{bmatrix} 1 & 0 \\ 0 & 1 \\ \frac{-I_1}{I_3} & \frac{-I_2}{I_3} \end{bmatrix}$$

 $T = \frac{1}{2}\dot{\boldsymbol{\theta}}^{T} M \dot{\boldsymbol{\theta}} = \frac{1}{2}\dot{\boldsymbol{\theta}}^{\prime T} C^{T} M C \dot{\boldsymbol{\theta}}^{\prime} = \frac{1}{2}\dot{\boldsymbol{\theta}}^{\prime T} M^{\prime} \dot{\boldsymbol{\theta}}^{\prime}$ $V = \frac{1}{2}\boldsymbol{\theta}^{T} K \boldsymbol{\theta} = \frac{1}{2}\boldsymbol{\theta}^{\prime T} C^{T} K C \boldsymbol{\theta}^{\prime} = \frac{1}{2}\boldsymbol{\theta}^{\prime T} K^{\prime} \boldsymbol{\theta}^{\prime}$



7.8 SYSTEMS ADMITTING RIGID-BODY MOTIONS

$$M' = C^{T} M C = \frac{1}{I_{3}} \begin{bmatrix} I_{1}(I_{1} + I_{3}) & I_{1}I_{2} \\ I_{1}I_{2} & I_{2}(I_{2} + I_{3}) \end{bmatrix}$$
$$K' = C^{T} K C =$$
$$\frac{1}{I_{3}^{2}} \begin{bmatrix} k_{1}I_{3}^{2} + k_{2}I_{1}^{2} & -k_{1}I_{3}^{2} + k_{2}I_{1}(I_{2} + I_{3}) \\ -k_{1}I_{3}^{2} + k_{2}I_{1}(I_{2} + I_{3}) & k_{1}I_{3}^{2} + k_{2}(I_{2} + I_{3})^{2} \end{bmatrix}$$
$$K' \Theta' = \omega^{2} M' \Theta'$$



7.8 SYSTEMS ADMITTING RIGID-BODY MOTIONS $\theta_1(t)$ $M' = \frac{1}{I} \begin{bmatrix} 2I^2 & I^2 \\ I^2 & 2I^2 \end{bmatrix} = I \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix}$ $K' = \frac{1}{I^2} \begin{bmatrix} 2kI^2 & kI^2 \\ kI^2 & 5kI^2 \end{bmatrix} = k \begin{bmatrix} 2 & 1 \\ 1 & 5 \end{bmatrix}$ $\theta_2(t)$ $\theta_{3}(t)$ GJ $\omega_1 = \sqrt{\frac{k}{I}}, \ \mathbf{\Theta}_1' = \begin{bmatrix} 1 \\ 0 \end{bmatrix} \qquad \omega_2 = \sqrt{\frac{3k}{I}}, \ \mathbf{\Theta}_2' = \begin{bmatrix} 0.5 \\ -1 \end{bmatrix}$

 $\boldsymbol{\Theta}_{0} = \begin{bmatrix} 1\\1\\1 \end{bmatrix} \qquad \boldsymbol{\Theta}_{1} = \begin{bmatrix} 1&0\\0&1\\-1&-1 \end{bmatrix} \begin{bmatrix} 1\\0 \end{bmatrix} = \begin{bmatrix} 1\\0\\-1 \end{bmatrix}$ $\boldsymbol{\Theta}_{2} = \begin{bmatrix} 1&0\\0&1\\-1&-1 \end{bmatrix} \begin{bmatrix} 0.5\\-1\\0.5 \end{bmatrix} = \begin{bmatrix} 0.5\\-1\\0.5 \end{bmatrix}$



7.Multi-Degree-of-Freedom Systems

7.1 Equations of Motion for Linear Systems

- **7.2** Flexibility and Stiffness Influence Coefficients
- **7.3** Properties of the Stiffness and Mass Coefficients

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Advanced Vibrations

Lecture 7

MODE 2

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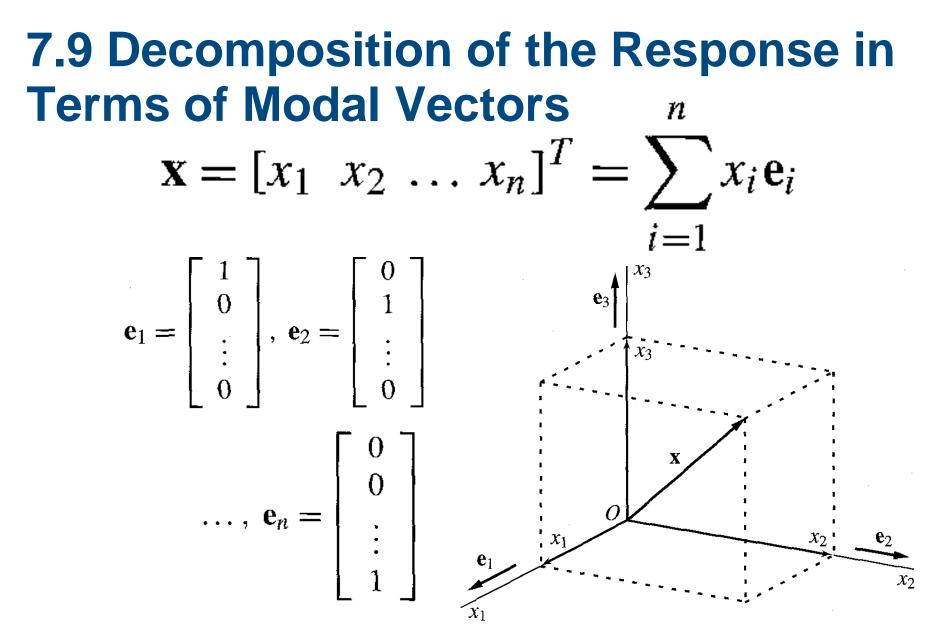
7.15 Response to External Excitations by Modal Analysis

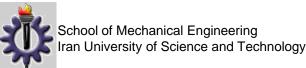
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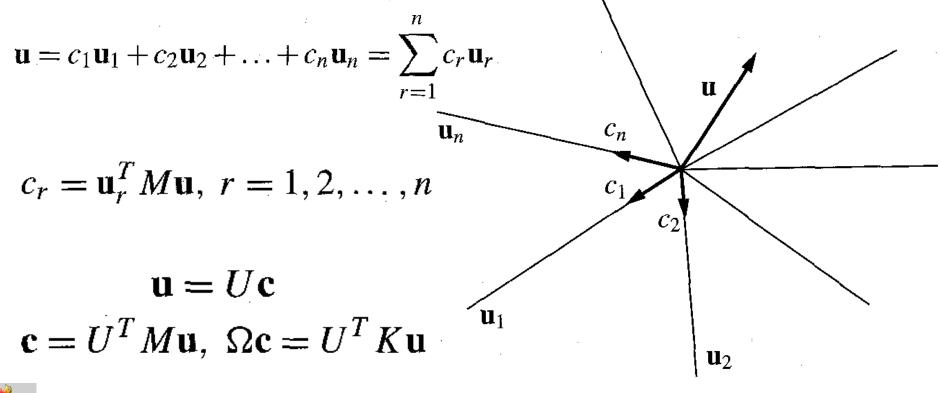




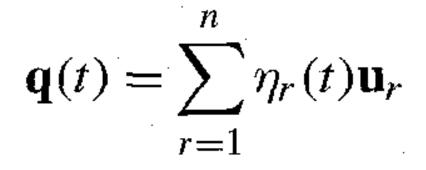


7.9 Decomposition of the Response in Terms of Modal Vectors

The modal vectors are orthonormal with respect to the mass matrix M, \searrow



$M\ddot{\mathbf{q}}(t) + K\mathbf{q}(t) = \mathbf{0}$



 $\eta_r(t) = \mathbf{u}_r^T M \mathbf{q}(t), \ \omega_r^2 \eta_r(t) = \mathbf{u}_r^T K \mathbf{q}(t), \ r = 1, 2, \dots, n$



$$\mathbf{q}(t) = \sum_{r=1}^{n} \eta_r(t) \mathbf{u}_r \quad \longrightarrow \mathbf{q}(t) = U \boldsymbol{\eta}(t)$$

Modal Coordinates

n

$$\begin{aligned} \ddot{\boldsymbol{\eta}}(t) + \Omega \boldsymbol{\eta}(t) &= \boldsymbol{0} \\ \ddot{\eta}_r(t) + \omega_r^2 \eta_r(t) &= 0, \ r = 1, 2, \dots, n \\ \eta_r(t) &= C_r \cos(\omega_r t - \phi_r) \\ &= \eta_r(0) \cos\omega_r t + \frac{\dot{\eta}_r(0)}{\omega_r} \sin\omega_r t, \end{aligned}$$



$$\eta_r(t) = \mathbf{u}_r^T M \mathbf{q}(0) \cos \omega_r t + \frac{1}{\omega_r} \mathbf{u}_r^T M \dot{\mathbf{q}}(0) \sin \omega_r t,$$

$$r = 1, 2, \dots, n$$

$\mathbf{q}(t) = \sum_{r=1}^{n} [\mathbf{u}_{r}^{T} M \mathbf{q}(0) \cos \omega_{r} t + \frac{1}{\omega_{r}} \mathbf{u}_{r}^{T} M \dot{\mathbf{q}}(0) \sin \omega_{r} t] \mathbf{u}_{r}$



We wish to demonstrate that each of the natural modes can be excited independently of the other;

$$\mathbf{q}(0) = \alpha \mathbf{u}_{s}, \ \dot{\mathbf{q}}(0) = \mathbf{0}$$

$$\mathbf{q}(t) = \sum_{n}^{n} [\mathbf{u}_{r}^{T} M \mathbf{q}(0) \cos \omega_{r} t + \frac{1}{\omega_{r}} \mathbf{u}_{r}^{T} M \dot{\mathbf{q}}(0) \sin \omega_{r} t] \mathbf{u}_{r}$$

$$\mathbf{q}(t) = \alpha \sum_{r=1}^{n} [\mathbf{u}_{r}^{T} M \mathbf{u}_{s} \cos \omega_{r} t] \mathbf{u}_{r}$$

$$= \alpha \sum_{r=1}^{n} \delta_{rs} \mathbf{u}_{r} \cos \omega_{r} t = \alpha \mathbf{u}_{s} \cos \omega_{s} t$$

7.11 Eigenvalue Problem in Terms of a Single Symmetric Matrix

$$K\mathbf{u} = \omega^2 M\mathbf{u}$$

$$M = LL^T \quad K\mathbf{u} = \omega^2 LL^T \mathbf{u}$$

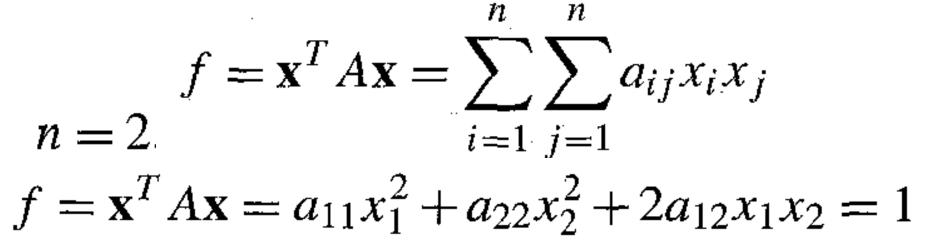
$$L^T \mathbf{u} = \mathbf{v}$$

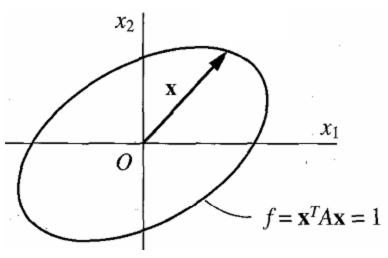
$$A\mathbf{v} = \lambda \mathbf{v}, \ \lambda = \omega^2$$

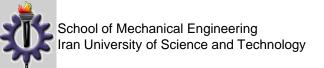
$$A = L^{-1} K (L^{-1})^T = A^T$$

$$\mathbf{v}_r = \delta_{rs}, \ \mathbf{v}_s^T A \mathbf{v}_r = \lambda_r \delta_{rs}, \ r, s = 1, \ 2, \dots, n$$

7.12 Geometric Interpretation of the Eigenvalue Problem



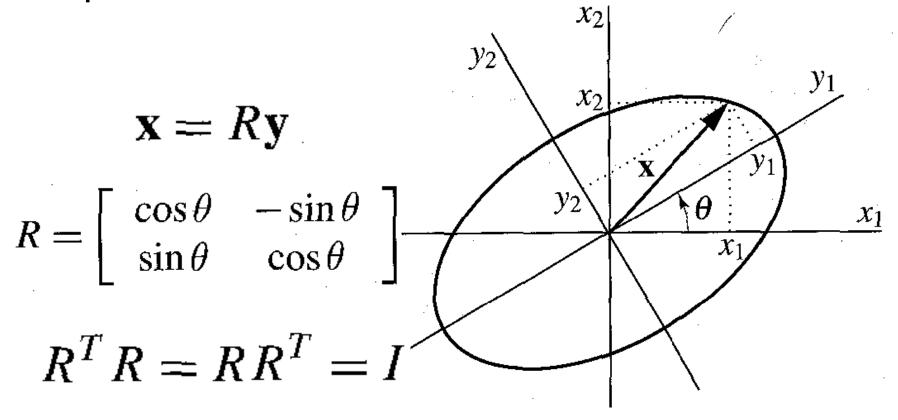




$$\nabla f = \begin{bmatrix} \frac{\partial f}{\partial x_1} \\ \frac{\partial f}{\partial x_2} \end{bmatrix} = 2 \begin{bmatrix} a_{11}x_1 + a_{12}x_2 \\ a_{12}x_1 + a_{22}x_2 \end{bmatrix}$$
$$= 2 \begin{bmatrix} a_{11} & a_{12} \\ a_{12} & a_{22} \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = 2A\mathbf{x}$$
$$\nabla f = 2\lambda \mathbf{x}$$
$$A\mathbf{x} = \lambda \mathbf{x}$$



Solving the eigenvalue problem by finding the principle axes of the ellipse.





$$f = \mathbf{x}^T A \mathbf{x} = \mathbf{y}^T R^T A R \mathbf{y} = \mathbf{y}^T D \mathbf{y} = 1$$

Transforming to canonical form implies elimination of cross products:

$$D = R^T A R = \text{diag}[d_1 \ d_2]$$
$$D = \Lambda, \ R = V$$

$$\begin{bmatrix} \lambda_1 & 0 \\ 0 & \lambda_2 \end{bmatrix} = \begin{bmatrix} \cos\theta & -\sin\theta \\ \sin\theta & \cos\theta \end{bmatrix}^T \begin{bmatrix} a_{11} & a_{12} \\ a_{12} & a_{22} \end{bmatrix} \begin{bmatrix} \cos\theta & -\sin\theta \\ \sin\theta & \cos\theta \end{bmatrix}$$

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 $\lambda_1 = a_{11}\cos^2\theta + 2a_{12}\sin\theta\cos\theta + a_{22}\sin^2\theta$

 $\lambda_2 = a_{11}\sin^2\theta - 2a_{12}\sin\theta\cos\theta + a_{22}\cos^2\theta$

 $0 = -(a_{11} - a_{22})\sin\theta\cos\theta + a_{12}(\cos^2\theta - \sin^2\theta)$

 $\sin 2\theta = 2\sin\theta\cos\theta$ and $\cos 2\theta = \cos^2\theta - \sin^2\theta$,

$$\tan 2\theta = \frac{2a_{12}}{a_{11} - a_{22}}$$



Obtaining the angle, one may calculate the eigenvalues and eigenvectors:

$$\lambda_1 = a_{11}\cos^2\theta + 2a_{12}\sin\theta\cos\theta + a_{22}\sin^2\theta$$
$$\lambda_2 = a_{11}\sin^2\theta - 2a_{12}\sin\theta\cos\theta + a_{22}\cos^2\theta$$

$$\mathbf{v}_1 = \begin{bmatrix} \cos\theta\\ \sin\theta \end{bmatrix}, \ \mathbf{v}_2 = \begin{bmatrix} -\sin\theta\\ \cos\theta \end{bmatrix}$$



Example: Solving the eigenvalue problem by finding the principal axes of the corresponding ellipse.

$$M = m \begin{bmatrix} 1 & 0 \\ 0 & 2 \end{bmatrix}, \quad K = \frac{T}{L} \begin{bmatrix} 2 & -1 \\ -1 & 3 \end{bmatrix}$$
$$A = \begin{bmatrix} 1 & 0 \\ 0 & 1/\sqrt{2} \end{bmatrix} \begin{bmatrix} 2 & -1 \\ -1 & 3 \end{bmatrix} \begin{bmatrix} 1 & 0 \\ 0 & 1/\sqrt{2} \end{bmatrix}$$



7.12 Geometric Interpretation of the **Eigenvalue Problem** $A = \begin{bmatrix} 2 & -1/\sqrt{2} \\ -1/\sqrt{2} & 3/2 \end{bmatrix} \qquad \lambda = \omega^2 m L/T$ $\tan 2\theta = \frac{2a_{12}}{a_{11} - a_{22}} \quad b = a_{12} = -\frac{1}{\sqrt{2}} \quad c = \frac{1}{2}(a_{11} - a_{22}) = \frac{1}{4}$ $\cos\theta = \left[\frac{1}{2} + \frac{c}{2(b^2 + c^2)^{1/2}}\right]^{1/2} = 0.816497$ $\sin\theta = \frac{\nu}{2(b^2 + c^2)^{1/2}\cos\theta} = -0.577350$



$$\lambda_{1} = a_{11} \cos^{2} \theta + 2a_{12} \sin \theta \cos \theta + a_{22} \sin^{2} \theta$$

$$= 2.5$$

$$\lambda_{2} = a_{11} \sin^{2} \theta - 2a_{12} \sin \theta \cos \theta + a_{22} \cos^{2} \theta$$

$$= 1$$

$$\mathbf{v}_{1} = \begin{bmatrix} \cos \theta \\ \sin \theta \end{bmatrix} = \begin{bmatrix} 0.816497 \\ -0.577350 \end{bmatrix},$$

$$\mathbf{v}_{2} = \begin{bmatrix} -\sin \theta \\ \cos \theta \end{bmatrix} = \begin{bmatrix} 0.577350 \\ 0.816497 \end{bmatrix}$$



7.Multi-Degree-of-Freedom Systems

7.1 Equations of Motion for Linear Systems

- **7.2** Flexibility and Stiffness Influence Coefficients
- **7.3** Properties of the Stiffness and Mass Coefficients

7.4 Lagrange's Equations Linearized about Equilibrium

7.5 Linear Transformations : Coupling

7.6 Undamped Free Vibration :The Eigenvalue Problem

7.7 Orthogonality of Modal Vectors

7.8 Systems Admitting Rigid-Body Motions

7.9 Decomposition of the Response in Terms of Modal Vectors

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- 7.15.1 Undamped systems
- 7.15.2 Systems with proportional damping

7.16 Systems with Arbitrary Viscous Damping

7.17 Discrete-Time Systems



Advanced Vibrations

Lecture 8

MODE 2

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UMASS LOWELL MODAL ANALYSIS and CONTROLS LABORATORY - Pete Avitabile and Fabio Piergentili

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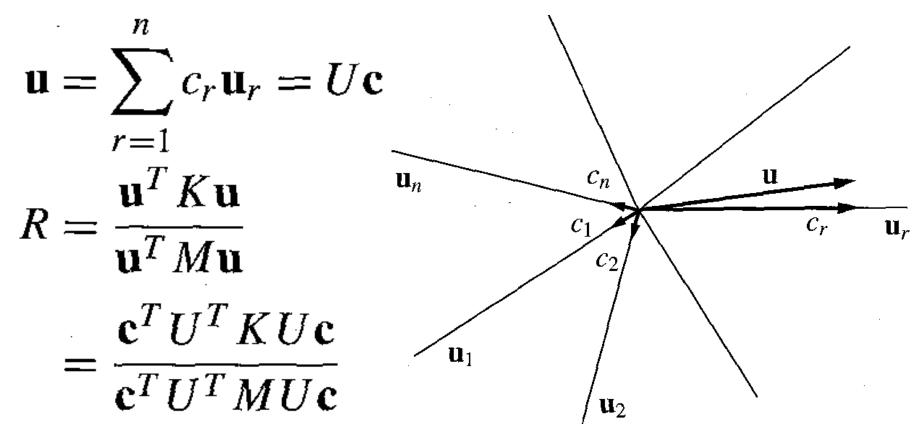


$$K\mathbf{u}_r = \lambda_r M \mathbf{u}_r, \ \lambda_r = \omega_r^2, \ r = 1, 2, \dots, n$$

$$\lambda_r = \omega_r^2 = \frac{\mathbf{u}_r^T K \mathbf{u}_r}{\mathbf{u}_r^T M \mathbf{u}_r}, \ r = 1, 2, \dots, n$$

 $R(\mathbf{u}) = \lambda = \omega^2 = \frac{\mathbf{u}^T K \mathbf{u}}{\mathbf{u}^T M \mathbf{u}}$ Rayleigh's quotient





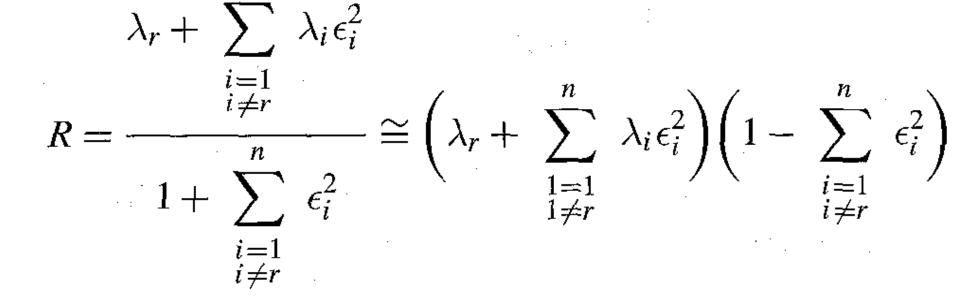


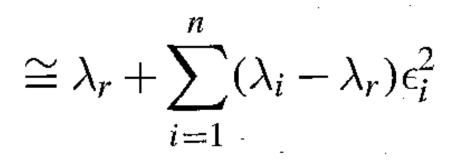
7.13 RAYLEIGH'S QUOTIENT AND ITS PROPERTIES $\sum_{i=1}^{n} \lambda_i c_i^2$

 $R = \frac{\mathbf{u}^T K \mathbf{u}}{\mathbf{u}^T M \mathbf{u}} = \frac{\mathbf{c}^T U^T K U \mathbf{c}}{\mathbf{c}^T U^T M U \mathbf{c}} = \frac{\mathbf{c}^T \Lambda \mathbf{c}}{\mathbf{c}^T \mathbf{c}} = \frac{\sum_{i=1}^{n} \lambda_i c_i}{\sum_{i=1}^{n} c_i^2}$ $U^T M U = I, \ U^T K U = \Lambda$

$$c_{i} = \epsilon_{i}c_{r}, \ i = 1, 2, \dots, n; \ i \neq r \qquad \begin{array}{c} \lambda_{r} + \sum_{\substack{i=1\\i \neq r}}^{n} \lambda_{i}\epsilon_{i}^{2} \\ R = \frac{1}{1 + \sum_{\substack{i=1\\i \neq r}}^{n} \epsilon_{i}^{2}} \end{array}$$









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Of special interest in vibrations is the fundamental frequency. n

$$R \cong \lambda_1 + \sum_{i=2}^{\infty} (\lambda_i - \lambda_1) \epsilon_i^2 \longrightarrow R \ge \lambda_1$$

Rayleigh's quotient is an upper bound for the lowest eigenvalue.

$$\lambda_1 = \min_{\mathbf{u}} R(\mathbf{u}) = \min_{\mathbf{u}} \frac{\mathbf{u}^T K \mathbf{u}}{\mathbf{u}^T M \mathbf{u}}$$



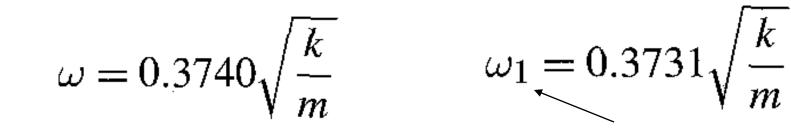
Example:

$$M = m \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 2 \end{bmatrix}, K = k \begin{bmatrix} 2 & -1 & 0 \\ -1 & 3 & -2 \\ 0 & -2 & 2 \end{bmatrix}$$

$$\mathbf{F} = c[m_1 \ m_2 \ m_3]^T = [1 \ 1 \ 2]^T \text{ Simulates gravity loading}$$
$$\mathbf{u} = \frac{1}{k} \begin{bmatrix} 2 & -1 & 0 \\ -1 & 3 & -2 \\ 0 & -2 & 2 \end{bmatrix}^{-1} \begin{bmatrix} 1 \\ 1 \\ 2 \end{bmatrix} = \frac{1}{k} \begin{bmatrix} 4 \\ 7 \\ 8 \end{bmatrix}$$



 $R = \omega^2 = \frac{\mathbf{u}^T K \mathbf{u}}{\mathbf{u}^T M \mathbf{u}} = \frac{27k}{193m} = 0.1399 \frac{k}{m}$



Exact solution

$$\frac{\omega - \omega_1}{\omega_1} = \frac{0.3740 - 0.3731}{0.3731} = 0.002412 = 0.2412\%$$



$$\mathbf{u} = \begin{bmatrix} 0.3522 \\ 0.6163 \\ 0.7044 \end{bmatrix}, \ \mathbf{u}_1 = \begin{bmatrix} 0.3309 \\ 0.6155 \\ 0.7152 \end{bmatrix}$$

$$\frac{\|\mathbf{u} - \mathbf{u}_1\|}{\|\mathbf{u}_1\|} = \sqrt{(\mathbf{u} - \mathbf{u}_1)^T (\mathbf{u} - \mathbf{u}_1)} = 0.0239 = 2.39\%$$



7.14 RESPONSE TO HARMONIC EXTERNAL EXCITATIONS $M\ddot{\mathbf{q}}(t) + C\dot{\mathbf{q}}(t) + K\mathbf{q}(t) = \mathbf{Q}(t)$ $\mathbf{Q}(t) = \mathbf{Q}_0 e^{i\,\alpha t}$ $\mathbf{q}(t) = \mathbf{q}_0 e^{i\,\alpha t}$ $(-\alpha^2 M + i\alpha C + K)\mathbf{q}_0 e^{i\alpha t} = \mathbf{Q}_0 e^{i\alpha t}$ $Z(i\alpha)\mathbf{q}_0 = \mathbf{Q}_0$ $Z(i\alpha) = -\alpha^2 M + i\alpha C + K$ $\mathbf{q}_0 = Z^{-1}(i\alpha)\mathbf{O}_0$



7.14 RESPONSE TO HARMONIC EXTERNAL EXCITATIONS $\mathbf{q}_0 = Z^{-1}(i\alpha)\mathbf{Q}_0$ $Z^{-1}(i\alpha) = G(i\alpha)$ $\mathbf{q}(t) = G(i\alpha)\mathbf{Q}_0 e^{i\alpha t}$

This approach is feasible only for systems with a small number of degrees of freedom.

For large systems, it becomes necessary to adopt an approach based on the idea of decoupling the equations of motion.



7.15 RESPONSE TO EXTERNAL EXCITATIONS BY MODAL ANALYSIS: Undamped systems

 $M\ddot{\mathbf{q}}(t) + K\mathbf{q}(t) = \mathbf{Q}(t)$ $K\mathbf{u} = \omega^2 M\mathbf{u} \quad U^T M U = I. \quad U^T K U = \Omega$ $\mathbf{q}(t) = \sum \eta_r(t) \mathbf{u}_r = U \boldsymbol{\eta}(t)$ r=1 $\ddot{\boldsymbol{\eta}}(t) + \Omega \boldsymbol{\eta}(t) = \mathbf{N}(t)$



7.15 RESPONSE TO EXTERNAL EXCITATIONS BY MODAL ANALYSIS

$\ddot{\boldsymbol{\eta}}(t) + \Omega \boldsymbol{\eta}(t) = \mathbf{N}(t)$ $\mathbf{N}(t) = U^T \mathbf{Q}(t)$ $\ddot{\eta}_r(t) + \omega_r^2 \eta_r(t) = N_r(t), \ r = 1, 2, \dots, n$

 $N_r(t) = \mathbf{u}_r^T \mathbf{Q}(t), \ r = 1, 2, \dots, n$



7.15 RESPONSE TO EXTERNAL EXCITATIONS BY MODAL ANALYSIS Harmonic excitation

$$\mathbf{Q}(t) = \mathbf{Q}_0 \cos \alpha t$$
$$N_r(t) = \mathbf{u}_r^T \mathbf{Q}_0 \cos \alpha t, \ r = 1, 2, \dots, n$$
$$\eta_r(t) = \frac{\mathbf{u}_r^T \mathbf{Q}_0}{\omega_r^2 - \alpha^2} \cos \alpha t, \ r = 1, 2, \dots, n$$
$$\mathbf{q}(t) = \sum_{r=1}^n \frac{\mathbf{u}_r^T \mathbf{Q}_0}{\omega_r^2 - \alpha^2} \mathbf{u}_r \cos \alpha t$$



7.15 RESPONSE TO EXTERNAL EXCITATIONS BY MODAL ANALYSIS: Transient Vibration

$$\eta_r(t) = \frac{1}{\omega_r} \int_0^t N_r(t-\tau) \sin \omega_r \tau \ d\tau, \ r = 1, 2, \dots, n$$

$$\mathbf{q}(t) = \sum_{r=1}^{n} \left[\frac{\mathbf{u}_{r}^{T}}{\omega_{r}} \int_{0}^{t} \mathbf{Q}(t-\tau) \sin \omega_{r} \tau \ d\tau \right] \mathbf{u}_{r}$$



7.15 RESPONSE TO EXTERNAL **EXCITATIONS BY MODAL ANALYSIS:** Systems admitting rigid-body modes $\ddot{\eta}_r(t) = N_r(t), \ r = 1, 2, \dots, i$ $\eta_r(t) = \int_0^t \left| \int_0^\tau N_r(\sigma) d\sigma \right| d\tau, \ r = 1, 2, \dots, i$ $\mathbf{q}(t) = \sum_{r=1}^{i} \mathbf{u}_r^T \int_0^t \left[\int_0^\tau \mathbf{Q}(\sigma) d\sigma \right] d\tau$ $+\sum_{r=i+1}^{n} \left[\frac{\mathbf{u}_{r}^{T}}{\omega_{r}} \int_{0}^{t} \mathbf{Q}(t-\tau) \sin \omega_{r} \tau \ d\tau \right] \mathbf{u}_{r}$

School of Mechanical Engineering Iran University of Science and Technology 7.15 RESPONSE TO EXTERNAL EXCITATIONS BY MODAL ANALYSIS: Systems with proportional damping

$$C = \alpha M + \beta K$$
$$U^{T} C U = U^{T} (\alpha M + \beta K) U =$$
$$\alpha U^{T} M U + \beta U^{T} K U = \alpha I + \beta \Omega$$

 $\ddot{\boldsymbol{\eta}}(t) + (\alpha I + \beta \Omega) \dot{\boldsymbol{\eta}}(t) + \Omega \boldsymbol{\eta}(t) = \mathbf{N}(t)$ $\Omega = \operatorname{diag}(\omega_1^2 \ \omega_2^2 \ \dots \ \omega_n^2)$ $\alpha + \beta \omega_r^2 = 2\zeta_r \omega_r, \ r = 1, 2, \dots, n$



7.15 RESPONSE TO EXTERNAL EXCITATIONS BY MODAL ANALYSIS: Harmonic excitation

 $\ddot{\eta}_r(t) + 2\zeta_r \omega_r \dot{\eta}_r(t) + \omega_r^2 \eta_r(t) = N_r(t), \ r = 1, 2, \dots, n$ $\mathbf{Q}(t) = \mathbf{Q}_0 e^{i\,\alpha t}$ $N_r(t) = \mathbf{u}_r^T \mathbf{Q}_0 e^{i\,\alpha t}, \ r = 1, 2, \dots, n$ $\mathbf{u}_r^T \mathbf{Q}_0$ $\eta_r(t)$ $\overline{\alpha^2 + i2\zeta_r\omega_r\alpha}$ $\frac{1}{\omega^2 - \alpha^2 + i2\zeta_r\omega_r\alpha}$ $i \alpha t$ $\mathbf{q}(t)$ • $\overline{\omega_r^2}$ -

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7.15 RESPONSE TO EXTERNAL EXCITATIONS BY MODAL ANALYSIS: Transient Vibration

$$\eta_r(t) = \frac{1}{\omega_{dr}} \int_0^t N_r(t-\tau) e^{-\zeta_r \omega_r \tau} \sin \omega_{dr} \tau \, d\tau,$$

$$\omega_{dr} = (1 - \zeta_r^2)^{1/2} \omega_r, \ r = 1, 2, \dots, n$$



7.Multi-Degree-of-Freedom Systems

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Advanced Vibrations

Lecture 9

MODE 2

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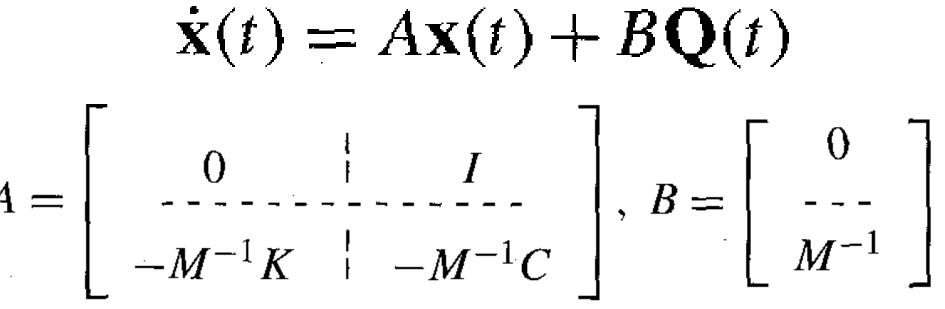
7.16 Systems with Arbitrary Viscous Damping

7.17 Discrete-Time Systems



7.16 SYSTEMS WITH ARBITRARY VISCOUS DAMPING

$\dot{\mathbf{q}}(t) = \dot{\mathbf{q}}(t)$ $\ddot{\mathbf{q}}(t) = -M^{-1}C\dot{\mathbf{q}}(t) - M^{-1}K\mathbf{q}(t) + M^{-1}\mathbf{Q}(t)$





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7.16 SYSTEMS WITH ARBITRARY VISCOUS DAMPING

Nonsymmetric $\mathbf{Q}(t) = \mathbf{0}$ $\dot{\mathbf{x}}(t) = A\mathbf{x}(t)$ $\mathbf{x}(t) = e$ $A\mathbf{x} = \lambda \mathbf{x}$ $A^T \mathbf{v} = \lambda \mathbf{y}$ $det[A - \lambda I] = det[A^T - \lambda I] = 0$

The eigenvalues/vectors are in general complex.



7.16 SYSTEMS WITH ARBITRARY VISCOUS DAMPING: Orthogonality

$$\mathbf{y}^T A = \lambda \mathbf{y}^T$$

Left eigenvectors

 $\mathbf{y}_j^T A \mathbf{x}_i = \lambda_i \mathbf{y}_j^T \mathbf{x}_i$

 $\mathbf{y}_{i}^{T} A \mathbf{x}_{i} = \lambda_{j} \mathbf{y}_{i}^{T} \mathbf{x}_{i}$



Right eigenvectors

$$A\mathbf{x}_i = \lambda_i \mathbf{x}_i, \ i = 1, 2, \dots, 2n$$
$$\mathbf{y}_j^T A = \lambda_j \mathbf{y}_j^T, \ j = 1, 2, \dots, 2n$$

 $(\lambda_i - \lambda_j) \mathbf{y}_j^T \mathbf{x}_i = 0$



7.16 SYSTEMS WITH ARBITRARY VISCOUS DAMPING

$$(\lambda_i - \lambda_j) \mathbf{y}_j^T \mathbf{x}_i = 0$$

$$\mathbf{y}_{j}^{T} \mathbf{x}_{i} = 0, \qquad \lambda_{i} \neq \lambda_{j}, \ i, j = 1, 2, \dots, 2n$$
$$\mathbf{y}_{j}^{T} A \mathbf{x}_{i} = 0, \qquad \lambda_{i} \neq \lambda_{j}, \ i, j = 1, 2, \dots, 2n$$

X2

 \mathbf{X}_1

The right eigenvectors \mathbf{x}_i are **biorthogonal** to the left eigenvectors \mathbf{y}_i .



T

Biorthonormality Relations

$$\mathbf{y}_j^T \mathbf{x}_i = \delta_{ij} \quad \mathbf{y}_j^T A \mathbf{x}_i = \lambda_i \delta_{ij}, \quad i, j = 1, 2, \dots, 2n$$

$$\begin{array}{c|c} Y^T X = I \\ Y^T = X^{-1} \\ XY^T = I \end{array} \quad \begin{array}{c|c} AX = X\Lambda \\ A^T Y = Y\Lambda \end{array} \quad \begin{array}{c|c} Y^T AX = \Lambda \\ A = X\Lambda Y^T \end{array}$$

The **bi-orthogonality** property forms **the basis for a modal analysis** for the response of systems with arbitrary viscous damping.



Assume an arbitrary 2n-dimensional state vector:

$$\mathbf{v} = X\mathbf{a}$$
$$\mathbf{a} = Y^T \mathbf{v} \qquad \Lambda \mathbf{a} = Y^T A \mathbf{v}$$

The expansion theorem forms the basis for a state space modal analysis: 2n

$$\mathbf{x}(t) = \xi_1(t)\mathbf{x}_1 + \xi_2(t)\mathbf{x}_2 + \dots + \xi_{2n}(t)\mathbf{x}_{2n} = \sum_{r=1}^{n} \xi_r(t)\mathbf{x}_r$$
$$= X\boldsymbol{\xi}(t)$$



$\dot{\mathbf{x}}(t) = A\mathbf{x}(t) + B\mathbf{Q}(t)$

$Y^T X \dot{\boldsymbol{\xi}}(t) = Y^T A X \boldsymbol{\xi}(t) + Y^T B \mathbf{Q}(t)$

 $\dot{\boldsymbol{\xi}}(t) = \Lambda \boldsymbol{\xi}(t) + \mathbf{n}(t)$

 $\mathbf{n}(t) = Y^T B \mathbf{Q}(t)$



7.16 SYSTEMS WITH ARBITRARY VISCOUS DAMPING: Harmonic Excitations

- - +

$$\mathbf{Q}(t) = \mathbf{Q}_{0}e^{i\alpha t}$$

$$n_{r}(t) = \mathbf{y}_{r}^{T}B\mathbf{Q}_{0}e^{i\alpha t}$$

$$r = 1, 2, \dots, 2n$$

$$\dot{\xi}_{r}(t) = \Xi_{r}(i\alpha)e^{i\alpha t}, \quad r = 1, 2, \dots, 2n$$

$$\dot{\xi}_{r}(t) = \lambda_{r}\xi_{r}(t) + n_{r}(t),$$

$$(i\alpha - \lambda_{r})\Xi_{r}(i\alpha)e^{i\alpha t} = \mathbf{y}_{r}^{T}B\mathbf{Q}_{0}e^{i\alpha t},$$

$$\Xi_{r}(i\alpha) = \frac{\mathbf{y}_{r}^{T}B\mathbf{Q}_{0}}{i\alpha - \lambda_{r}}$$



7.16 SYSTEMS WITH ARBITRARY VISCOUS DAMPING: Harmonic Excitations

$$\xi_r(t) = \frac{\mathbf{Y}_r^T B \mathbf{Q}_0}{i\alpha - \lambda_r} e^{i\alpha t}$$
$$\mathbf{x}(t) = \sum_{r=1}^{2n} \frac{\mathbf{y}_r^T B \mathbf{Q}_0}{i\alpha - \lambda_r} \mathbf{x}_r e^{i\alpha t}$$



7.16 SYSTEMS WITH ARBITRARY VISCOUS DAMPING: Arbitrary Excitations

$$\dot{\xi}_r(t) = \lambda_r \xi_r(t) + n_r(t),$$

$$s \Xi_r(s) - \xi_r(0) = \lambda_r \Xi_r(s) + N_r(s),$$

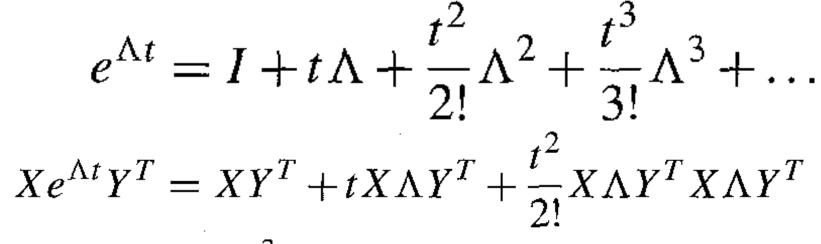
$$\xi_r(0) = y_r^T \mathbf{x}(0), r = 1, 2, \dots, 2n$$

$$\begin{aligned} \xi_r(t) &= \mathcal{L}^{-1} \Xi_r(s) = \\ e^{\lambda_r t} \xi_r(0) + \int_0^t e^{\lambda_r (t-\tau)} n_r(\tau) d\tau, \end{aligned}$$



$$\boldsymbol{\xi}(t) = e^{\Lambda t} \boldsymbol{\xi}(0) + \int_0^t e^{\Lambda(t-\tau)} \mathbf{n}(\tau) d\tau$$
$$\mathbf{n}(t) = Y^T B \mathbf{Q}(t) \qquad \boldsymbol{\xi}(0) = Y^T \mathbf{x}(0)$$
$$\mathbf{x}(t) = X e^{\Lambda t} \boldsymbol{\xi}(0) + \int_0^t X e^{\Lambda(t-\tau)} \mathbf{n}(\tau) d\tau$$
$$\mathbf{x}(t) = X e^{\Lambda t} Y^T \mathbf{x}(0) + \int_0^t X e^{\Lambda(t-\tau)} Y^T B \mathbf{Q}(\tau) d\tau$$



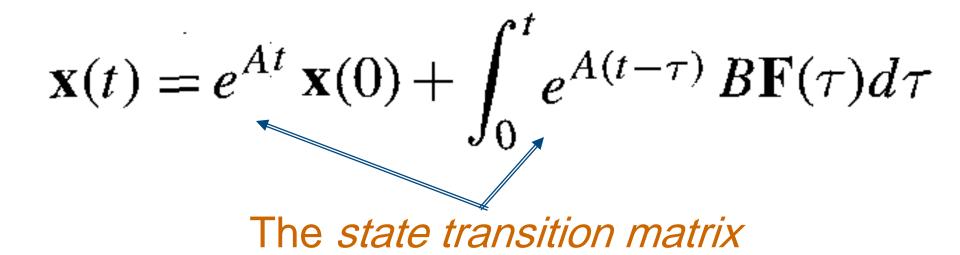


$$+\frac{t^3}{3!}X\Lambda Y^T X\Lambda Y^T X\Lambda Y^T + \dots$$

 $= I + tA + \frac{t^2}{2!}A^2 + \frac{t^3}{3!}A^3 + \dots = e^{At}$



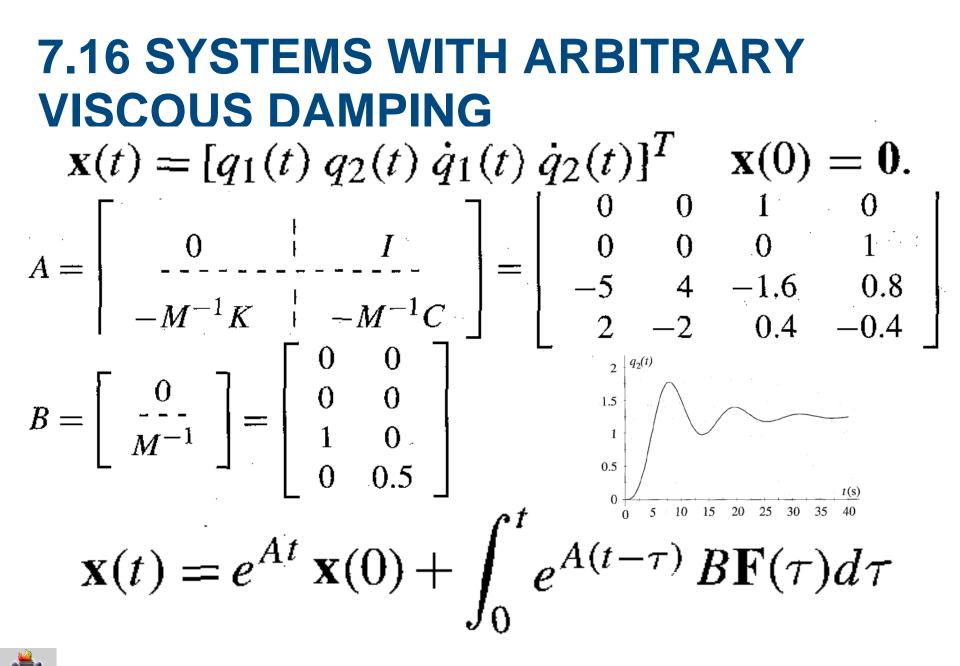
$$\mathbf{x}(t) = X e^{\Lambda t} Y^T \mathbf{x}(0) + \int_0^t X e^{\Lambda(t-\tau)} Y^T B \mathbf{Q}(\tau) d\tau$$





Example 7.12. Determine the response of the system to the excitation: $Q_1(t) = 0, \ Q_2(t) = Q_0[tw(t) - (t-4)w(t-4)]$ $M = \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} = \begin{bmatrix} 1 & 0 \\ 0 & 2 \end{bmatrix}$ $C = \begin{vmatrix} c_1 + c_2 & -c_2 \\ -c_2 & c_2 \end{vmatrix} = \begin{vmatrix} 1.6 & -0.8 \\ -0.8 & 0.8 \end{vmatrix}$ $Q_1(t)$ $Q_2(t)$ $K = \begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix} = \begin{bmatrix} 5 & -4 \\ -4 & 4 \end{bmatrix}$ $q_1(t)$ $q_2(t)$ k_2 m_1 m_2 C_1 C_{2}





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7.Multi-Degree-of-Freedom Systems

7.1 Equations of Motion for Linear Systems

- **7.2** Flexibility and Stiffness Influence Coefficients
- **7.3** Properties of the Stiffness and Mass Coefficients

7.4 Lagrange's Equations Linearized about Equilibrium

7.5 Linear Transformations : Coupling

7.6 Undamped Free Vibration :The Eigenvalue Problem

7.7 Orthogonality of Modal Vectors

7.8 Systems Admitting Rigid-Body Motions

7.9 Decomposition of the Response in Terms of Modal Vectors

7.10 Response to Initial Excitations by Modal Analysis

7.11 Eigenvalue Problem in Terms of a Single Symmetric Matrix

7.12 Geometric Interpretation of the Eigenvalue Problem

7.13 Rayleigh's Quotient and Its Properties

7.14 Response to Harmonic External Excitations

7.15 Response to External Excitations by Modal Analysis

- 7.15.1 Undamped systems
- 7.15.2 Systems with proportional damping

7.16 Systems with Arbitrary Viscous Damping

7.17 Discrete-Time Systems

