

OPTIMIZATION OF LARGE-SCALE TRUSS STRUCTURES USING MODIFIED CHARGED SYSTEM SEARCH

A. Kaveh^{1,*},[†] and S. Talatahari²

¹*Centre of Excellence for Fundamental Studies in Structural Engineering, Iran University of
Science and Technology, Narmak, Tehran-16, Iran*

²*Marand Faculty of Engineering, University of Tabriz, Tabriz, Iran Iran*

ABSTRACT

Optimal design of large-scale structures is a rather difficult task and the computational efficiency of the currently available methods needs to be improved. In view of this, the paper presents a modified Charged System Search (CSS) algorithm. The new methodology is based on the combination of CSS and Particle Swarm Optimizer. In addition, in order to improve optimization search, the sequence of tasks entailed by the optimization process is changed so that the updating of the design variables can directly be performed after each movement. In this way, the new method acts as a single-agent algorithm while preserving the positive characteristics of its original multi-agent formulation.

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1. INTRODUCTION

Design optimization of structures is a challenging task for the designers and engineers who attempt to minimize the cost of the structure yet satisfying design constraints posed by the standard codes of practice. In general, this is a difficult problem since the relationships between design variables and optimization constraints are not straightforward. This becomes more complicated when the size of the problem increases, where the search space has a large

*Corresponding author: A. Kaveh, Centre of Excellence for Fundamental Studies in Structural Engineering, Iran University of Science and Technology, Narmak, Tehran-16, Iran

[†]E-mail address: alikaveh@iust.ac.ir

size and a great number of design constraints must be controlled.

Despite difficulties involved in obtaining optimum designs of large-scale structures, engineers have always attempted to have optimal structures [1]. The tools to fulfill this goal are optimization methods. However, classical optimization algorithms [2-8] for large-scale problems need many powerful computational systems. Furthermore, many of such algorithms are known as local optimizer and their final results cannot be considered as the global optimum. Contrary to mathematical programming algorithms, there are meta-heuristic algorithms which are often stochastic algorithms and can efficiently explore the search space of the large-scale problems.

Charged System Search (CSS) is a new meta-heuristic optimization algorithm inspired by the governing laws of electrical physics and the Newtonian mechanics [9,10]. In electrical physics, the electric charge can generate the electric field and exerts a force on other electrically charged objects. The electric field surrounding a point charge is specified by the laws of Coulomb and Gauss. Utilizing these principles, the CSS algorithm defines a number of solution candidates each of which is called charged particle (CP) and is treated as a charged sphere. Each CP can exert an electrical force on the other agents (CPs). These forces can change the position of other CPs according to the Newton's second law. Finally, considering the Newtonian mechanics, the new positions of CPs are determined. Application of the CSS on some structural problems reveals the good performance of this new method [10-12].

This paper presents a modified CSS algorithm for optimal design of large-scale truss structures. The proposed method combines the CSS with Particle Swarm Optimization (PSO) instead of the standard CSS formulation. Hybridization of CSS with PSO has recently been proposed by the present authors in Ref. [13]. The novelty of the present study is that the updating process of the algorithm's memory is changed directly after each movement. This is done in order to improve the performance of the optimization algorithm. Consequently, the new method works as a single agent algorithm yet preserving the positive characteristics of its original multi-agent formulation. Two examples of large-scale truss structures to be designed for minimum volume are considered in order to demonstrate the capability of the new algorithm in solving large-scale structural optimization problems.

2. FORMULATION OF THE TRUSS-STRUCTURE DESIGN OPTIMIZATION PROBLEM

The general formulation of the weight minimization problem for a truss structure is as follows:

$$\begin{aligned}
 \text{minimize} \quad & W(\{x\}) = \sum_{i=1}^n \gamma_i \cdot A_i \cdot L_i \\
 \text{subject to:} \quad & \delta_{\min} \leq \delta_i \leq \delta_{\max} \quad i = 1, 2, \dots, m \\
 & \sigma_{\min} \leq \sigma_i \leq \sigma_{\max} \quad i = 1, 2, \dots, n \\
 & \sigma_i^b \leq \sigma_i \leq 0 \quad i = 1, 2, \dots, nc \\
 & A_{\min} \leq A_i \leq A_{\max} \quad i = 1, 2, \dots, ng
 \end{aligned} \tag{1}$$

where $W(\{x\})$ = weight of the structure; n = number of members making up the structure; m = number of nodes; nc = number of elements subjected to compression; ng = number of groups (number of design variables); γ_i = material density of member i ; L_i = length of member i ; A_i = cross-sectional area of member i chosen between A_{min} and A_{max} ; min = lower bound and max = upper bound; σ_i and δ_i = stress and nodal deflection, respectively; σ_i^b = allowable buckling stress in member i when it is subjected to compression.

3. CHARGED SYSTEM SEARCH ALGORITHM

3.1. Description of the standard Charged Search System

The Charged System Search (CSS) is a population-based search approach, where each agent (CP) is considered as a charged sphere with radius a , having a uniform volume charge density which can produce an electric force on the other CPs. The force magnitude for a CP located in the inside of the sphere is proportional to the separation distance between the CPs, while for a CP located outside the sphere it is inversely proportional to the square of the separation distance between the particles. The resultant forces or acceleration and the motion laws determine the new location of the CPs. The pseudo-code for the CSS algorithm can be summarized as follows:

Step 1: Initialization. The initial positions of CPs are determined randomly in the search space and the initial velocities of charged particles are assumed to be zero. The values of the fitness function for the CPs are determined and the CPs are sorted in an increasing order. A number of the first CPs and their related values of the fitness function are saved in a memory, so called charged memory (CM).

Step 2: Determination of forces on CPs. The force vector is calculated for each CP as

$$\mathbf{F}_j = \sum_{i, i \neq j} \left(\frac{q_i}{a^3} r_{ij} \cdot i_1 + \frac{q_i}{r_{ij}^2} \cdot i_2 \right) a r_{ij} p_{ij} (\mathbf{X}_i - \mathbf{X}_j) \begin{cases} j = 1, 2, \dots, N \\ i_1 = 1, i_2 = 0 \Leftrightarrow r_{ij} < a \\ i_1 = 0, i_2 = 1 \Leftrightarrow r_{ij} \geq a \end{cases} \quad (2)$$

where \mathbf{F}_j is the resultant force acting on the j th CP; N is the number of CPs. The magnitude of charge for each CP (q_i) is defined considering the quality of its solution as

$$q_i = \frac{fit(i) - fitworst}{fitbest - fitworst}, \quad i = 1, 2, \dots, N \quad (3)$$

where $fitbest$ and $fitworst$ are the best and the worst fitness of all particles, respectively; $fit(i)$ represents the fitness of the agent i ; and N is the total number of CPs. The separation distance r_{ij} between two charged particles is defined as follows:

$$r_{ij} = \frac{\|\mathbf{X}_i - \mathbf{X}_j\|}{\|(\mathbf{X}_i + \mathbf{X}_j)/2 - \mathbf{X}_{best}\| + \varepsilon} \quad (4)$$

where \mathbf{X}_i and \mathbf{X}_j are respectively the positions of the i th and j th CPs, \mathbf{X}_{best} is the position of the best current CP, and ε is a small positive number. Here, p_{ij} is the probability of moving each CP towards the others and is obtained using the following function:

$$p_{ij} = \begin{cases} 1 & \frac{fit(i) - fit_{best}}{fit(j) - fit(i)} > rand \vee fit(j) > fit(i) \\ 0 & \text{else} \end{cases} \quad (5)$$

In Eq. (2), ar_{ij} indicates the kind of force and is defined as

$$ar_{ij} = \begin{cases} +1 & rand < 0.8 \\ -1 & \text{otherwise} \end{cases} \quad (6)$$

where $rand$ represents a random number.

Step 3: Solution construction. Each CP moves to the new position and the new velocity is calculated as

$$\mathbf{X}_{j,new} = rand_{j1} \cdot k_a \cdot \mathbf{F}_j + rand_{j2} \cdot k_v \cdot \mathbf{V}_{j,old} + \mathbf{X}_{j,old} \quad (7)$$

$$\mathbf{V}_{j,new} = \mathbf{X}_{j,new} - \mathbf{X}_{j,old} \quad (8)$$

where k_a is the acceleration coefficient; k_v is the velocity coefficient to control the influence of the previous velocity; and $rand_{j1}$ and $rand_{j2}$ are two random numbers uniformly distributed in the range (0,1).

Step 4: Updating process. If a new CP exits from the allowable search space, a harmony search-based handling approach [9] is used to correct its position. In addition, if some new CP vectors are better than the worst ones in the **CM**, these are replaced by the worst ones in the **CM**.

Step 5: Termination criterion control. Steps 2-4 are repeated until a termination criterion is satisfied.

3.2. Particle Swarm Optimization added to Charged Search System

The Particle Swarm Optimization (PSO) utilizes a velocity term which is a combination of the previous velocity, \mathbf{V}_i^k , the movement in the direction of the local best (i.e. the best visited position by the particle itself), \mathbf{P}_i^k , the movement in the direction of the global best (i.e. the best

visited position of all the particles in its neighborhood), \mathbf{P}_g^k . In the present hybrid algorithm [13], the advantage of the PSO consisting of utilizing the local best and the global best is added to the CSS algorithm. The charged memory (**CM**) for the hybrid algorithm is treated as the local best in the PSO, and the **CM** updating process is defined as follows:

$$\mathbf{CM}_{i,new} = \begin{cases} \mathbf{CM}_{i,old} & W(\mathbf{X}_{i,new}) \geq W(\mathbf{CM}_{i,old}) \\ \mathbf{X}_{i,new} & W(\mathbf{X}_{i,new}) < W(\mathbf{CM}_{i,old}) \end{cases} \quad (9)$$

in which the first term identifies that when the new position is not better than the previous one, the updating will not be performed, while when the new position is better than the so far stored good position, the new solution vector is replaced. Considering the above mentioned new charged memory, the electric forces generated by agents are modified as

$$\mathbf{F}_j = \sum_{i \in S_1} \left(\frac{q_i}{a^3} r_{ij} \cdot i_1 + \frac{q_i}{r_{ij}^2} \cdot i_2 \right) ar_{ij} p_{ij} (\mathbf{CM}_{i,old} - \mathbf{X}_j) + \sum_{i \in S_2} \left(\frac{q_i}{a^3} r_{ij} \cdot i_1 + \frac{q_i}{r_{ij}^2} \cdot i_2 \right) ar_{ij} p_{ij} (\mathbf{X}_i - \mathbf{X}_j) \quad (10)$$

where subtitles S_1 and S_2 denote two sets of the numbers which determine the number of the agents utilized to calculate the resultant force by employing the agents sorted in the **CM** and the current agents positions, respectively. If the coefficient k_i is defined as

$$k_i = \left(\frac{q_i}{a^3} r_{ij} \cdot i_1 + \frac{q_i}{r_{ij}^2} \cdot i_2 \right) ar_{ij} p_{ij} \quad (11)$$

Then the resultant force formulae can be simplified as

$$\mathbf{F}_j = k_1 (\mathbf{CM}_{g,old} - \mathbf{X}_j) + k_2 (\mathbf{CM}_{j,old} - \mathbf{X}_j) + \sum_{i \in S_1} k_i (\mathbf{CM}_{i,old} - \mathbf{X}_j) + \sum_{i \in S_2} k_i (\mathbf{X}_i - \mathbf{X}_j) \quad (12)$$

where the subtitle g denotes the number of the stored so far good position among all CPs. Therefore the first term directs the agents towards the global best position. When $i = j$, then the $\mathbf{CM}_{i,old}$ is treated similar to \mathbf{P}_i^k in the PSO as considered in the second term of the above equation. This will direct the agents towards the local best. The sets S_1 and S_2 are defined as follows [13]:

$$S_1 = \{t_1, t_2, \dots, t_n \mid q(t) > q(j), j = 1, 2, \dots, N, j \neq i, g\} \quad (13)$$

$$S_2 = S - S_1 \quad (14)$$

where S_1 defines a set of n agents taken from **CM** and utilized in Eq. (12). If the set S includes all agents, the set S_2 will be the set of currently updated agents used to direct agent j . In addition, in the early optimization cycles n is set to zero and is then linearly increased to N towards the end of the optimization process.

3.3. Enhancing the proposed CSS-based algorithm

As mentioned before, CSS is a population-based algorithm. For multi-agent methods, the updating process is performed after all agents have created their solutions. Similarly, for the CSS algorithm, when the calculations of the amount of forces are completed for all CPs and the new locations of agents are determined, the **CM** updating is performed. In the present case, it is assumed that after creating just one solution, all updating processes are performed. In this way, the new position of each agent can affect on the moving of the subsequent CPs while in the standard CSS unless an iteration is completed, the new positions cannot be utilized [12]. Due to using the information obtained by CPs immediately after creation, this modification will enhance the final algorithm. Figure 1 shows the flowchart of the final CSS-based algorithm.

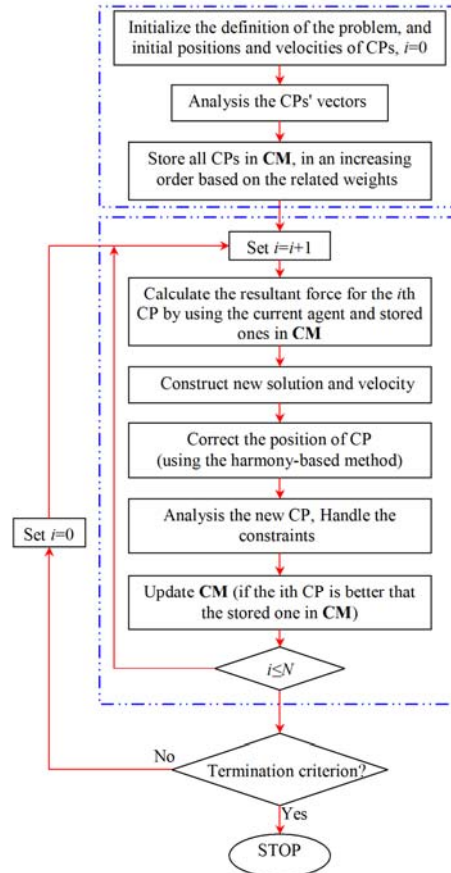


Figure 1. Flowchart for the new CSS-based algorithm.

4. DESIGN EXAMPLES

Two large-scale truss structures are selected from [7] to verify the efficiency of the new optimization algorithm. The structural material is steel with elastic modulus $E = 68,948$ MPa. The allowable stress for each member is 172.375 MPa. The stopping criterion is selected in a way that the new algorithm can reach better results than those obtained by Wang and Arora [7]. As soon as such a design is achieved, the searching process is stopped. The algorithms are coded in Matlab and a direct stiffness method is used for the analyses and designs.

4.1. A 35-storey space tower

The 35-storey space tower consists of 1,262 members and 936 degrees of freedom [7]. The base of the structure in the X–Y plane is as shown in Figure 2. The entire structure consists of three different sections from top to the bottom. Seventy-two design variables are used to represent the cross-sectional areas of 72 member groups, employing symmetry of the structure. The lower and upper bounds on the cross-sectional areas are $6.4516E-4$ m² and $6.4516E-2$ m² (1.0 and 100 in²), respectively. The loading on the structure consists of downward vertical loads and horizontal loads as follows:

- A. The vertical loads are given as 13.3446 kN at each node in the first section, 26.6892 kN at each node in the second section, and 40.0338 kN at each node in the third section.
- B. The horizontal loads are given as 4.4482 kN in the X direction at each node on the left side, 4.4482 kN in the X direction at each node on the right side.
- C. The horizontal loads are given as 4.4482 kN in the Y direction at each node on the back side, 4.4482 kN in the Y direction at each node on the front side.

The displacement constraints are 0.508 m in the X, Y and Z directions for the four nodes on the top level (about 1/250 of the height). Four loading conditions are considered, which consist of different combination of the lateral loads and vertical loads acting on the structure:

1. Loading condition A alone.
2. Loading conditions A and B acting together.
3. Loading conditions A and C acting together.
4. Loading conditions A, B and C acting together

The corresponding weight obtained by the new algorithm is equal to 51.88 m³ while the best result reported by [7] is 52.06 m³. The required number for convergence in the present algorithm is 17,500 analyses. Figure 3 shows the best and average convergence history for the results of the modified CSS. The plot of average convergence history is obtained using the information of 20 runs with different primary seeds. The difference between the convergence curves recorded respectively for the best design and the results average is small and this confirms the robustness of the proposed algorithm. Figure 4 shows the variation of some design variables for the optimization run corresponding to the best design overall. Whilst in the first iterations the values of selected variables change considerably because of the high exploration power of the algorithm, oscillations reduce as the optimization process

progresses and then become marginal in the final iterations. This indicates that a local search is performed towards the end of the optimization process.

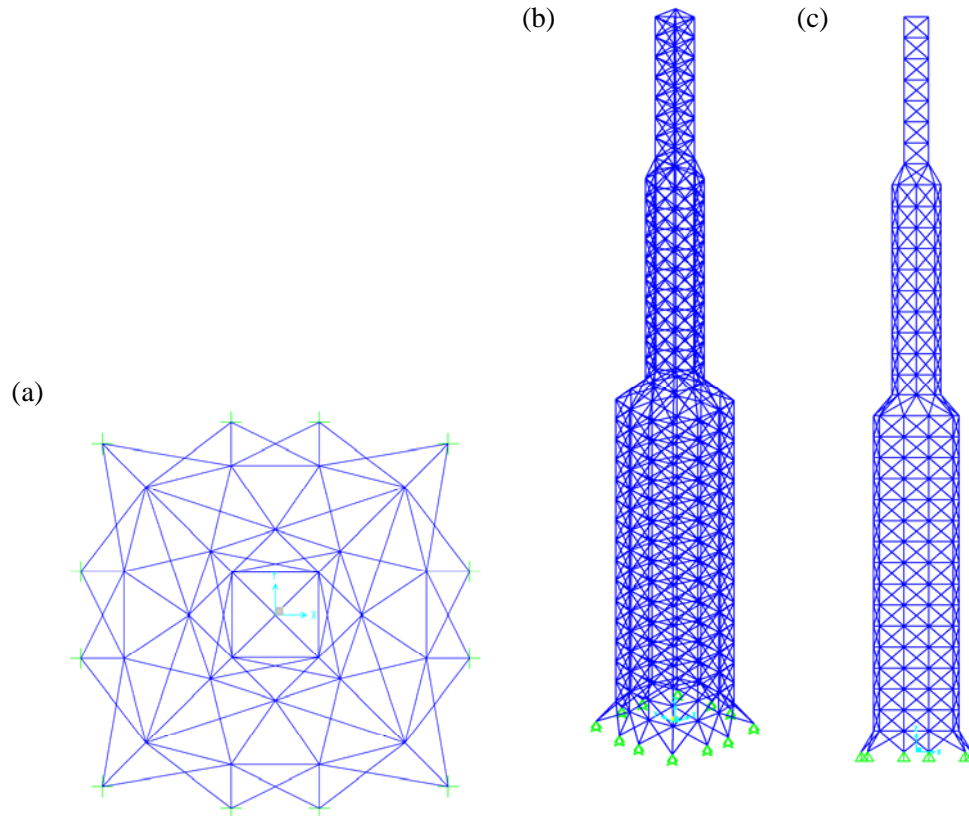


Figure 2. Schematic of the 35-storey space tower: a) Top view of the tower; b) 3D view of the structure; (c) Side view.

Another interesting finding appears from the comparison of cross-sectional areas with the total length of elements belonging to the corresponding groups. It means that when for some design variables the sum of element lengths is large, the selected area often becomes small. Conversely, for groups with a small sum of lengths, large values are assigned to the cross-sectional area in order to increase the stiffness of the structure. In this way, by multiplying small areas (large areas) with large sum of element lengths (small ones), the optimum design corresponding to a small volume can be obtained. Although this is not a general rule, and it is not even true for some variables, the variables with large amount of length sum have often tendency to select weak cross sections.

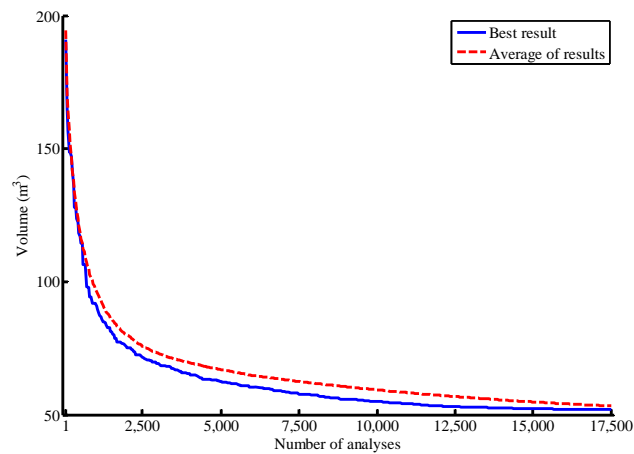


Figure 3. Convergence curves corresponding to the best design and the results average obtained for the 35-storey space tower

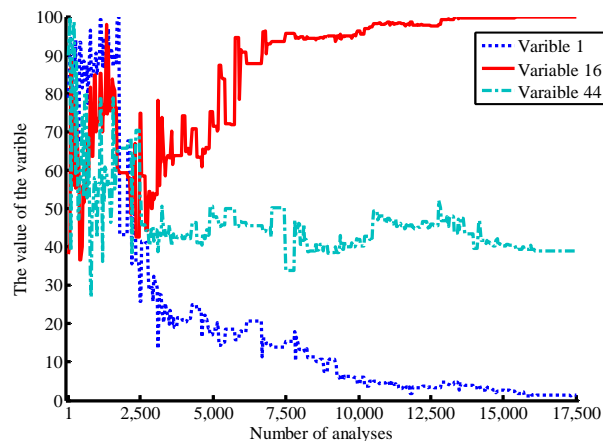


Figure 4. Convergence history for sizing variables 1, 16 and 44 corresponding to the best design obtained for the 35-storey space tower.

4.2. A 62-story space tower

The 62-storey space truss tower shown in Figure 6 consists of 4666 members and 2940 degrees of freedom. This structure is a variant of the 35-storey space tower considered in the previous example [7]. There are 238 design variables, representing different member groups as shown in Figure 6 [7]. The lower and upper bounds on the cross-sectional areas are $6.4516E-4 \text{ m}^2$ and 0.193548 m^2 (1.0 and 300 in^2), respectively. The loads on the structure are as follows:

- A. The vertical loads are given as 26.6892 kN at each node.
- B. The horizontal loads are given as 4.4482 kN in the X direction at each node on the left side, 4.4482 kN in the X direction at each node on the right side.
- C. The horizontal loads are given as 4.4482 kN in the Y direction at each node on the back side, 4.4482 kN in the Y direction at each node on the front side.

The displacement constraints are 0.90678 m in the X, Y and Z directions for the four nodes on the top level (about 1/250 of the height). Three loading conditions are considered, which consist of different combination of the lateral loads and vertical loads acting on the structure:

1. Loading condition A alone.
2. Loading conditions A and B acting together.
3. Loading conditions A, B and C acting together

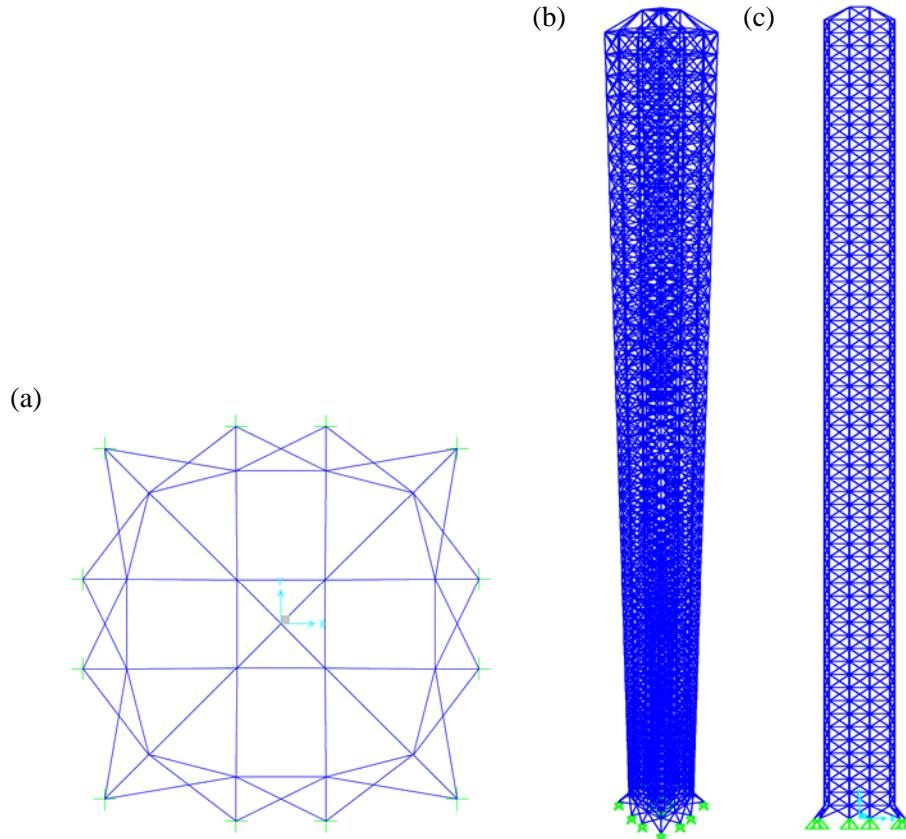


Figure 5. Schematic of the 62-storey space tower: a) Top view of the tower; b) 3D view of the structure; (c) Side view.

The optimum design of the 62-story is obtained after 25,000 analyses by using the modified CSS algorithm. The corresponding minimum volume is 349.3 m^3 . The optimum designs obtained by using SAND formulations had the volume of 350.5 m^3 [7]. As shown in Figure 7, it can be concluded that the global searching level of the new algorithm is completed after 18,500 analyses where its best so far result is 10% higher than the final optimum design. Table 1 summarizes the optimal design for the new algorithm. The global sway at the top of the tower is the active constraint where it is very close to its maximum allowable value. Though ignoring the stress constraints may lead to smaller weights, however, these constraints must also be considered during the optimization process. Figure 8 shows the final values of the variables

number 21, 2, 10, and 15 in the 20 runs. Almost all the runs converge to a determinable small domain, and it can be concluded that the final best optimum result of these variables will be in these domains.

Table 1. Optimized cross-sectional areas for the 62-storey space tower

No. of group	Area (cm ²)	No. of group	Area (cm ²)	No. of group	Area (cm ²)	No. of group	Area (cm ²)	No. of group	Area (cm ²)	No. of group	Area (cm ²)	No. of group	Area (cm ²)
1	6.452	35	6.452	69	6.452	103	1935.48	137	140.579	171	46.131	205	31.905
2	6.452	36	6.452	70	1935.48	104	639.920	138	57.468	172	48.789	206	6.452
3	892.042	37	1935.48	71	1327.16	105	77.484	139	60.680	173	6.452	207	21.628
4	1931.20	38	1935.48	72	88.219	106	74.434	140	6.478	174	6.452	208	6.452
5	6.452	39	80.524	73	81.173	107	6.452	141	8.645	175	6.452	209	6.452
6	1935.48	40	87.745	74	6.452	108	23.024	142	6.452	176	6.452	210	6.529
7	6.452	41	6.452	75	38.567	109	6.452	143	6.452	177	6.452	211	6.452
8	6.452	42	52.417	76	6.452	110	6.452	144	6.452	178	6.452	212	6.452
9	6.452	43	6.452	77	6.452	111	6.452	145	6.452	179	6.452	213	194.097
10	1370.763	44	6.452	78	6.452	112	6.452	146	6.452	180	842.124	214	6.452
11	6.452	45	6.452	79	6.452	113	6.452	147	1651.55	181	6.452	215	25.804
12	1225.06	46	6.452	80	6.452	114	1935.48	148	67.129	182	44.703	216	27.931
13	6.452	47	6.452	81	1935.48	115	424.997	149	56.296	183	44.845	217	6.452
14	6.452	48	1935.48	82	1082.345	116	75.535	150	51.999	184	6.477	218	23.340
15	1935.48	49	1793.11	83	85.219	117	68.514	151	6.4798	185	9.022	219	6.452
16	1935.48	50	84.928	84	71.339	118	6.452	152	6.475	186	6.452	220	6.452
17	25.972	51	91.362	85	6.452	119	18.056	153	6.475	187	6.475	221	6.452
18	12.44	52	6.452	86	33.767	120	6.452	154	6.452	188	6.477	222	21.124
19	23.260	53	47.206	87	6.452	121	6.452	155	6.497	189	6.452	223	20.056
20	974.290	54	6.452	88	6.452	122	6.452	156	6.474	190	6.480	224	72.682
21	216.528	55	6.452	89	6.452	123	6.452	157	6.475	191	596.237	225	6.452
22	53.036	56	6.452	90	6.452	124	6.452	158	1402.36	192	6.475	226	18.239
23	9.309	57	6.452	91	6.475	125	1935.48	159	21.590	193	41.610	227	17.357
24	6.452	58	6.452	92	1935.48	126	256.643	160	52.830	194	40.471	228	6.452
25	6.452	59	1935.48	93	842.935	127	64.732	161	52.195	195	6.475	229	26.987
26	1935.48	60	1572.18	94	81.171	128	66.3102	162	6.452	196	14.202	230	6.452
27	1935.48	61	81.362	95	74.630	129	6.452	163	6.452	197	6.452	231	6.452
28	73.607	62	85.490	96	6.485	130	13.534	164	6.452	198	6.452	232	6.452
29	53.670	63	6.452	97	29.297	131	6.475	165	6.452	199	6.452	233	214.581
30	6.452	64	42.633	98	6.526	132	6.452	166	6.452	200	6.452	234	27.862
31	197.526	65	6.452	99	6.452	133	6.491	167	6.452	201	6.452	235	6.480
32	72.905	66	6.452	100	6.452	134	6.452	168	6.452	202	377.734	236	52.421
33	32.060	67	6.452	101	6.452	135	6.452	169	1121.19	203	6.452	237	12.948
34	10.706	68	6.452	102	6.452	136	1858.62	170	6.452	204	33.441	238	6.522

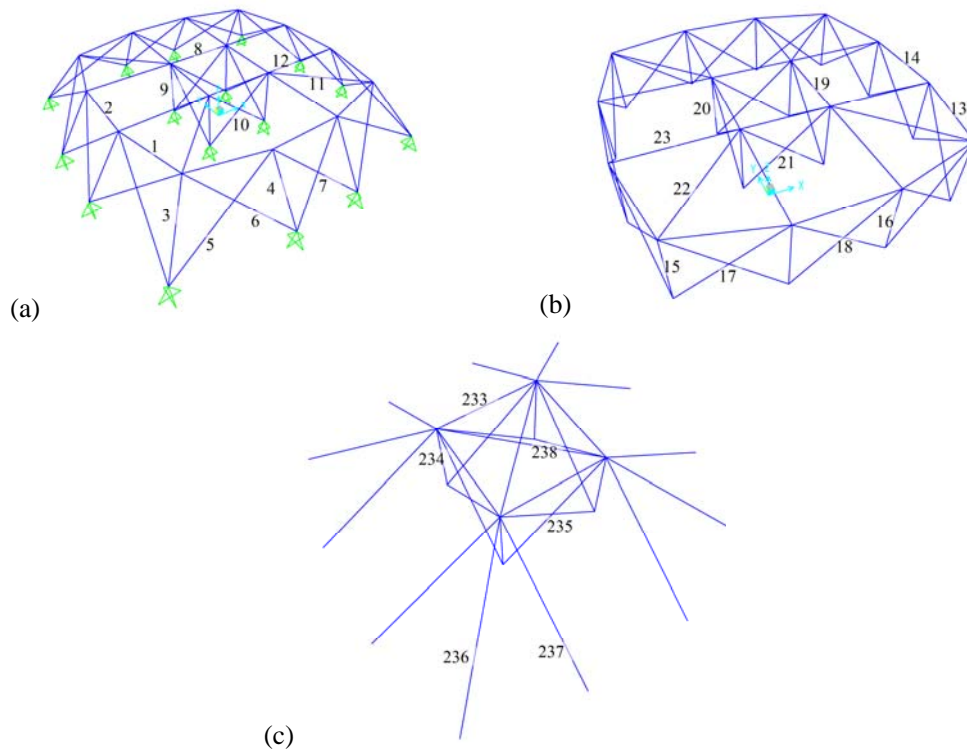


Figure 6. Member groups of 62-story space tower: (a) first storey; (b) second storey; and (c) top storey.

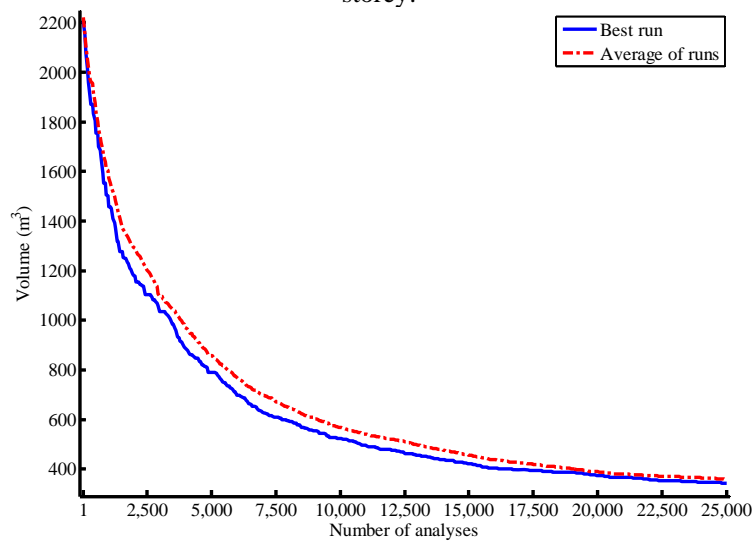


Figure 7. Convergence curves corresponding to the best design and the results average obtained for the 62-story space tower

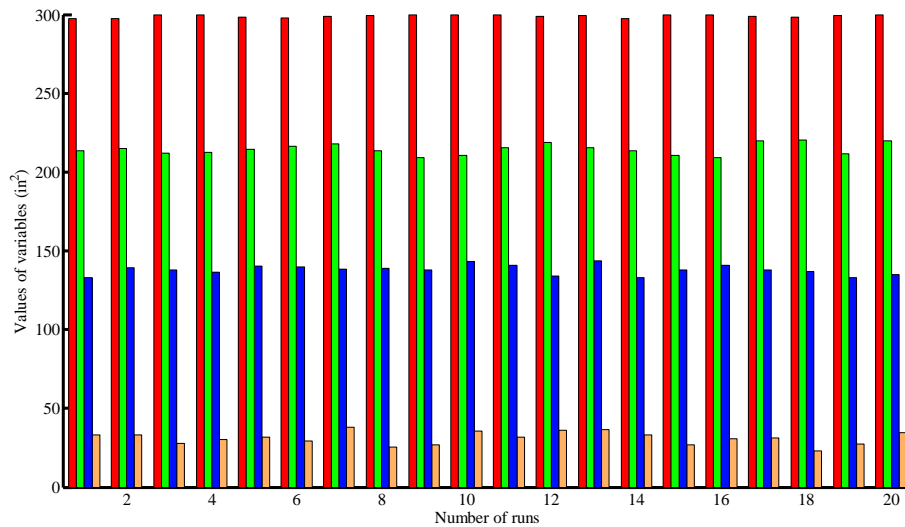


Figure 8. Values of design variables 21, 2, 10 and 15 obtained in the 20 optimization runs.

5. CONCLUDING REMARKS

This paper presented a modified Charged System Search algorithm for large-scale truss optimization problems. These problems are very complicated because of the presence of large search spaces as well as it is necessary to handle a large number of optimization constraints. Furthermore, it is a matter of fact that analyzing large-scale structures is computationally difficult. Therefore, the use of adaptive algorithms (rather than their original forms) seems to be the most convenient approach to optimum design of large-scale truss structures.

In view of this, a powerful algorithm is developed in this research by combining CSS and PSO. Furthermore, the design variables updating process is modified. The new algorithm in the CSS formulation introduces the PSO concept of local best and global best. Therefore, the charged memory is treated as the local best and redefined considering this point. The expression of the electric forces generated by each CP is modified in order to include the effect of the local best and global best points and the other agents. Finally, all updating processes are performed after creating just one solution. Hence the new position of each agent affects the movement of the subsequent CPs. Optimization results obtained by the modified CSS algorithm for two large-scale truss structures show that the proposed formulation can easily solve large-scale problems and requires small computational effort to find optimal designs.

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